

Bridge Load Rating Report Summary



Bridge ID: L15672000+02001

Feature Intersected: Swan River

Date Submitted 12-5-2014

Live Load Configuration Considered: HS20-44, Type 3, Type 3-3, Type 3S2

Bridge Description:

Steel through truss with a wood deck supported on steel stringers and steel floorbeams.

Inventory Rating Factor

R.F. =

Controlling Element: Exterior Stringer S8 (Simple Span) - Flexure

Operating Rating Factor

R.F. =

Controlling Element: Exterior Stringer S8 (Simple Span) - Flexure

Documented Assumptions:

- This Load Rating Analysis was performed for members only. No connections (including truss pin connections) were analyzed.
- Per MBE Table 6B.5.2.1-1, steel is Grade 30 based on year built of 1911.
- The end posts were the only top chord members accessible for measurement. Three of the four end posts have 0.25-inch cover plates, and one has a 0.375-inch cover plate. The Structure Inventory and Appraisal Sheet (1978) incorrectly shows that all top chords have 0.375-inch cover plates. All top chord members are modeled with 0.25-inch cover plates in the BrR model.
- When calculating the net area of riveted truss members, 0.75-inch rivet diameters were assumed.
- Member BC1 has 1/16-inch of section loss due to corrosion per the MDT Initial Assessment Form (2012). The corrosion was accounted for in the member's net area.

- The deck is made of timber planks. Since BrR does not support wood decks on truss bridges, the deck in the truss system superstructure is modeled as concrete with a thickness producing an equivalent weight of the actual wood deck.
- An additional superstructure definition was created to rate the wood deck only. In the deck model superstructure, the deck was given wet condition factors per the MDT Initial Assessment Form (2010). Douglas Fir-Larch No. 1 & Better deck planks were assumed. The shear stress factor C_H was input as 2.0 for the plank deck, since any splits or checks will be oriented parallel to the load direction and will not affect the shear strength of the member.
- Due to vehicular damage causing loss of straightness, the assumed compression capacity of member VW3L has been reduced to 60% of full capacity per engineering judgment. The MDT Fracture Critical Inspection (2010) states that counter members are “loose” and some eyes have forge cracks. To account for these imperfections, the tension capacity of members DW1, DW2, DW3, DW4, DW5, DW6, VW1, and VW6 has been reduced to 90% of full capacity per engineering judgment. An additional superstructure definition was created with the capacities of these truss members reduced. The capacities were reduced using the “MemberOfInterest” command’s “OverrideCapacity” subcommand in the truss input.
- An 8-inch utility pipe (water/sewer) runs the full length of the bridge and is hung from Stringer 2 near the floorbeams. The pipe weight was calculated assuming a full pipe, and point loads were applied to the stringers near the floorbeams.
- A built-up conduit box of 2x wood runs the full length of the bridge and is supported by the southeast ends of the floorbeams. The conduit weight was calculated and point loads were applied to the southeast ends of the floorbeams.
- The exterior stringer is a C8x11.5. Since BrR does not support channel stringers, the exterior stringers are modeled with the S6x12.5 because it has similar cross sectional properties as the C8x11.5.
- The undersides of the interior truss spans were not accessible for inspection. Based on what was visible from the end spans at the river banks, it is assumed that truss bays 1, 4, and 7 have simple span stringers and bays 2-3 and 5-6 have continuous (2-span) stringers.
- Per phone conversation with Amanda Jackson on 1/3/2014, the strength of the walkway is not to be analyzed. Also, no pedestrian live load was applied to the bridge or walkway, as the full pedestrian load occurring simultaneously with the largest truck load is unlikely. Only the dead load of the walkway was applied to the bridge. The walkway dead load was accordingly applied to the SE (Upstream) Truss and Stringers 6, 7, and 8. With the walkway cantilevering past the exterior stringer, an uplift reaction caused by the dead load of the walkway is produced at Stringer 6. Stringer 6 is assumed to have a connection to the walkway beams that is capable of supporting an uplift reaction.



- The controlling rating factor is for exterior stringer S8 in the single span condition. This element is controlling because of the added dead load from the cantilevered walkway on the southeast side of the truss.
- The rating factor presented in this Report Summary is based on the assumption that the wood deck does not act as a diaphragm at the midspan of the stringer. Per MDT Chapter 8 - Bridge Load Rating and Posting section 8.2.6.2.3, the deck only acts as a diaphragm if the girders are not twisting due to overload. Per MDT Initial Assessment Form comment dated 6/16/2004, “some stringers show some minor twisting.” Since the bridge is currently posted at 3 tons, the twisting in the stringers is possibly due to overload. Therefore, the deck should not be assumed to act as a diaphragm.

The presence of a diaphragm has significant effects on the stringers’ rating factors. If a diaphragm is assumed to be present, the inventory rating factor for member S8 (and the controlling rating factor for the bridge) increases from 0.050 to 0.156. However, based on MDT’s load rating protocol and MDT’s bridge inspection notes, the deck is not assumed to act as a diaphragm.

L15672000+02001
 Swan River (Truss)
 Summary of Deck, Stringer, Floorbeam, and Truss Rating Factors

Deck

Analysis Results - Deck

Report Type: Rating Results Summary

Lane/Impact Loading Type: As Requested Detailed

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating	Rating Factor	Location	Limit State	Impact	Lane
HS 20-44	Axle Load	ASD	Inventory	22.25	0.618		Flexure - One-lane	As Requested	As Requested
HS 20-44	Axle Load	ASD	Operating	29.62	0.823		Flexure - One-lane	As Requested	As Requested
Type 3	Axle Load	ASD	Inventory	25.76	1.030		Flexure - One-lane	As Requested	As Requested
Type 3	Axle Load	ASD	Operating	34.30	1.372		Flexure - One-lane	As Requested	As Requested
Type 3-3	Axle Load	ASD	Inventory	43.39	1.085		Flexure - One-lane	As Requested	As Requested
Type 3-3	Axle Load	ASD	Operating	57.77	1.444		Flexure - One-lane	As Requested	As Requested
Type 3S2	Axle Load	ASD	Inventory	40.13	1.115		Flexure - One-lane	As Requested	As Requested
Type 3S2	Axle Load	ASD	Operating	53.42	1.484		Flexure - One-lane	As Requested	As Requested

Madero(ASD/LRFD) - Version 1.02.05 - Sep. 11, 2012

Analysis Preference Setting: None

Close

Stringers - With the deck acting as a diaphragm

Exterior Stringers

S1 (Simple Span, C-beam)

Analysis Results - Stringer 1

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	13.52	0.375	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	22.57	0.627	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	9.90	0.275	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	16.53	0.459	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	8.46	0.338	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	14.13	0.565	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	16.44	0.411	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	27.46	0.686	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	13.37	0.371	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	22.32	0.620	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001|
Analysis Preference Setting: None

Close

S8 (Simple Span, C-beam)

Analysis Results - Stringer 8

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	7.68	0.213	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	12.83	0.356	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	5.63	0.156	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	9.40	0.261	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	4.81	0.192	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	8.03	0.321	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	9.34	0.234	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	15.60	0.390	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	7.60	0.211	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	12.68	0.352	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001|
Analysis Preference Setting: None

Close

S1 (2 Span, C-beam)

Analysis Results - Stringer 1

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	16.49	0.458	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	27.54	0.765	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	12.93	0.359	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	21.59	0.600	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	11.38	0.455	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	19.01	0.760	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	22.12	0.553	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	36.94	0.923	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	17.98	0.499	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	30.03	0.834	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S8 (2 Span, C-beam)

Analysis Results - Stringer 8

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	9.37	0.260	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	15.65	0.435	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	7.35	0.204	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	12.27	0.341	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	6.96	0.278	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	11.62	0.465	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	12.90	0.322	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	21.54	0.538	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	12.16	0.338	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	20.31	0.564	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested

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Analysis Preference Setting: None

Close

Interior Stringers

S2 (Simple Span, End Span W-beam)

Analysis Results - Stringer 2

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	10.91	0.303	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	18.23	0.506	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	7.99	0.222	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	13.35	0.371	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	6.78	0.271	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	11.32	0.453	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	13.17	0.329	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	22.00	0.550	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	10.71	0.297	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	17.88	0.497	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S3 (Simple Span, End Span W-beam)

Analysis Results - Stringer 3

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	13.49	0.375	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	22.53	0.626	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	9.88	0.274	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	16.50	0.458	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	8.41	0.336	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	14.04	0.562	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	16.34	0.408	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	27.29	0.682	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	13.28	0.369	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	22.18	0.616	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S6 (Simple Span, End Span W-beam)

Analysis Results - Stringer 6

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	15.05	0.418	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	25.14	0.698	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	11.03	0.306	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	18.42	0.512	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	9.34	0.374	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	15.60	0.624	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	18.15	0.454	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	30.31	0.758	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	14.75	0.410	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	24.64	0.684	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S7 (Simple Span, End Span W-beam)

Analysis Results - Stringer 7

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	10.57	0.294	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	17.65	0.490	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	7.74	0.215	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	12.93	0.359	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	6.61	0.264	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	11.04	0.442	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	12.84	0.321	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	21.44	0.536	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	10.44	0.290	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	17.43	0.484	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S2 (Simple Span, Middle Span S-beam)

Analysis Results - Stringer 2

Report Type: Rating Results Summary

Lane/Impact Loading Type: As Requested Detailed

Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	13.61	0.378	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	22.73	0.631	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	9.97	0.277	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	16.65	0.462	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	8.45	0.338	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	14.12	0.565	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	16.43	0.411	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	27.43	0.686	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	13.35	0.371	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	22.30	0.619	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S3 (Simple Span, Middle Span S-beam)

Analysis Results - Stringer 3

Report Type: Rating Results Summary

Lane/Impact Loading Type: As Requested Detailed

Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	16.69	0.464	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	27.88	0.774	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	12.23	0.340	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	20.42	0.567	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	10.40	0.416	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	17.37	0.695	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	20.20	0.505	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	33.74	0.843	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	16.42	0.456	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	27.43	0.762	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S6 (Simple Span, Middle Span S-beam)

Analysis Results - Stringer 6

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	18.26	0.507	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	30.49	0.847	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	13.37	0.371	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	22.33	0.620	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	11.33	0.453	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	18.92	0.757	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	22.01	0.550	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	36.76	0.919	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	17.89	0.497	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	29.88	0.830	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S7 (Simple Span, Middle Span S-beam)

Analysis Results - Stringer 7

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	13.26	0.368	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	22.15	0.615	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	9.71	0.270	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	16.22	0.451	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	8.28	0.331	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	13.83	0.553	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	16.09	0.402	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	26.88	0.672	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	13.08	0.363	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	21.85	0.607	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S2 (2 Span, S-beam)

Analysis Results - Stringer 2

Report Type: Rating Results Summary
 Lane/Impact Loading Type: As Requested Detailed
 Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	16.63	0.462	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	27.77	0.771	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	12.85	0.357	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	21.46	0.596	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	11.01	0.440	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	18.39	0.736	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	21.39	0.535	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	35.73	0.893	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	17.39	0.483	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	29.04	0.807	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
 Analysis Preference Setting: None

Close

S3 (2 Span, S-beam)

Analysis Results - Stringer 3

Report Type: Rating Results Summary
 Lane/Impact Loading Type: As Requested Detailed
 Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	20.37	0.568	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	34.02	0.945	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	15.49	0.430	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	25.87	0.719	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	13.28	0.531	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	22.17	0.887	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	25.79	0.645	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	43.07	1.077	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	20.97	0.582	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	35.02	0.973	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
 Analysis Preference Setting: None

Close

S6 (2 Span, S-beam)

Analysis Results - Stringer 6

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	22.28	0.619	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	37.20	1.033	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	16.27	0.452	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	27.17	0.755	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	13.94	0.558	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	23.28	0.931	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	27.09	0.677	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	45.23	1.131	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	22.02	0.612	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	36.77	1.021	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S7 (2 Span, S-beam)

Analysis Results - Stringer 7

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	16.18	0.450	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	27.03	0.751	17.00	1 - (100.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	12.65	0.351	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	21.13	0.587	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	10.84	0.434	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	18.10	0.724	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	21.06	0.527	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	35.17	0.879	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	17.12	0.476	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	28.59	0.794	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

Stringers - Without the deck acting as a diaphragm

Exterior Stringers

S1 (Simple Span, C-beam)

Analysis Results - Stringer 1

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	8.31	0.231	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	13.88	0.386	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	6.09	0.169	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	10.17	0.282	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	5.20	0.208	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	8.69	0.348	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	10.11	0.253	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	16.88	0.422	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	8.22	0.228	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	13.72	0.381	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S8 (Simple Span, C-beam)

Analysis Results - Stringer 8

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	2.48	0.069	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	4.13	0.115	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	1.81	0.050	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	3.03	0.084	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	1.55	0.062	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	2.59	0.104	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	3.01	0.075	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	5.03	0.126	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	2.45	0.068	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	4.09	0.114	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S1 (2 Span, C-beam)

Analysis Results - Stringer 1

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	12.04	0.335	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	20.11	0.559	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	8.66	0.241	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	14.47	0.402	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	7.43	0.297	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	12.40	0.496	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	14.43	0.361	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	24.09	0.602	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	11.73	0.326	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	19.59	0.544	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S8 (2 Span, C-beam)

Analysis Results - Stringer 8

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	8.01	0.223	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	13.38	0.372	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	5.77	0.160	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	9.63	0.267	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	4.94	0.198	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	8.25	0.330	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	9.60	0.240	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	16.03	0.401	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	7.80	0.217	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	13.03	0.362	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

Interior Stringers

S2 (Simple Span, End Span W-beam)

Analysis Results - Stringer 2

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	5.37	0.149	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	8.97	0.249	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	3.94	0.109	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	6.57	0.183	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	3.34	0.134	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	5.58	0.223	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	6.49	0.162	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	10.85	0.271	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	5.28	0.147	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	8.82	0.245	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S3 (Simple Span, End Span W-beam)

Analysis Results - Stringer 3

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	6.91	0.192	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	11.54	0.320	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	5.06	0.141	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	8.45	0.235	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	4.33	0.173	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	7.22	0.289	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	8.40	0.210	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	14.03	0.351	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	6.83	0.190	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	11.41	0.317	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S6 (Simple Span, End Span W-beam)

Analysis Results - Stringer 6

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	8.47	0.235	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	14.15	0.393	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	6.21	0.172	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	10.36	0.288	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	5.26	0.210	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	8.78	0.351	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	10.22	0.255	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	17.06	0.427	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	8.30	0.231	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	13.87	0.385	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S7 (Simple Span, End Span W-beam)

Analysis Results - Stringer 7

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	5.02	0.140	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	8.39	0.233	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	3.68	0.102	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	6.15	0.171	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	3.15	0.126	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	5.25	0.210	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	6.11	0.153	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	10.21	0.255	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	4.97	0.138	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	8.30	0.230	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S2 (Simple Span, Middle Span S-beam)

Analysis Results - Stringer 2

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	9.64	0.268	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	16.10	0.447	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	7.06	0.196	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	11.80	0.328	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	5.99	0.240	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	10.01	0.400	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	11.64	0.291	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	19.45	0.486	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	9.47	0.263	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	15.81	0.439	10.20	1 - (60.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S3 (Simple Span, Middle Span S-beam)

Analysis Results - Stringer 3

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	11.98	0.333	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	20.01	0.556	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	8.78	0.244	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	14.66	0.407	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	7.48	0.299	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	12.48	0.499	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	14.52	0.363	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	24.25	0.606	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	11.81	0.328	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	19.72	0.548	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S6 (Simple Span, Middle Span S-beam)

Analysis Results - Stringer 6

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	13.54	0.376	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	22.62	0.628	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	9.92	0.276	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	16.57	0.460	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	8.41	0.336	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	14.04	0.562	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	16.33	0.408	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	27.27	0.682	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	13.28	0.369	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	22.17	0.616	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S7 (Simple Span, Middle Span S-beam)

Analysis Results - Stringer 7

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	9.29	0.258	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	15.52	0.431	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	6.81	0.189	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	11.37	0.316	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	5.82	0.233	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	9.72	0.389	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	11.31	0.283	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	18.88	0.472	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	9.19	0.255	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	15.35	0.426	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S2 (2 Span, S-beam)

Analysis Results - Stringer 2

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	12.97	0.360	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	21.66	0.602	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	9.33	0.259	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	15.58	0.433	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	7.99	0.320	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	13.35	0.534	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	15.53	0.388	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	25.94	0.649	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	12.63	0.351	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	21.09	0.586	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S3 (2 Span, S-beam)

Analysis Results - Stringer 3

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	15.72	0.437	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	26.26	0.729	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	11.31	0.314	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	18.89	0.525	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	9.69	0.388	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	16.19	0.647	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	18.83	0.471	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	31.45	0.786	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	15.31	0.425	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	25.57	0.710	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S6 (2 Span, S-beam)

Analysis Results - Stringer 6

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	16.80	0.467	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	28.06	0.779	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	12.09	0.336	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	20.19	0.561	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	10.36	0.414	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	17.30	0.692	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	20.13	0.503	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	33.61	0.840	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	16.36	0.454	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	27.32	0.759	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

S7 (2 Span, S-beam)

Analysis Results - Stringer 7

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	12.69	0.353	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	21.19	0.589	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	9.13	0.254	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	15.25	0.424	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	7.82	0.313	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	13.07	0.523	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	15.20	0.380	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	25.39	0.635	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	12.36	0.343	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	20.64	0.573	6.80	1 - (40.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

Floorbeams

FB2

Analysis Results - Floorbeam 2

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	15.66	0.435	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	26.15	0.726	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	14.71	0.409	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	24.57	0.682	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	12.65	0.506	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	21.13	0.845	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	25.32	0.633	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	42.29	1.057	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	18.93	0.526	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	31.61	0.878	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

FB3

Analysis Results - Floorbeam 3

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	13.07	0.363	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	21.83	0.606	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	10.37	0.288	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	17.31	0.481	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	9.89	0.395	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	16.51	0.660	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	19.26	0.482	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	32.17	0.804	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	14.84	0.412	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	24.78	0.688	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

FB4

Analysis Results - Floorbeam 4

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Location (ft)	Location Span-(%)	Limit State	Impact	Lane
HS 20-44	Lane	LFD	Inventory	15.82	0.439	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	26.41	0.734	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Inventory	14.86	0.413	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	24.81	0.689	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	12.78	0.511	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	21.34	0.854	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	25.58	0.639	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	42.72	1.068	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	19.12	0.531	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	31.92	0.887	8.50	1 - (50.0)	Design Flexure - Steel	As Requested	As Requested

AASHTO LFR Engine Version 6.6.0.3001
 Analysis Preference Setting: None

Close

Truss

NW Truss (Downstream)

Analysis Results - NW Truss (Downstream)

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Element Name	Limit State	Impact	Lane
HS 20-44	Axle Load	LFD	Inventory	15.71	0.436	VW6	AXIAL-TENSION	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	26.23	0.729	VW6	AXIAL-TENSION	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	13.36	0.535	VW6	AXIAL-TENSION	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	22.32	0.893	VW6	AXIAL-TENSION	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	23.07	0.577	DW5	AXIAL-TENSION	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	38.52	0.963	DW5	AXIAL-TENSION	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	19.87	0.552	VW6	AXIAL-TENSION	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	33.18	0.922	VW6	AXIAL-TENSION	As Requested	As Requested
HS 20-44	Lane	LFD	Inventory	16.63	0.462	VW1	AXIAL-TENSION	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	27.78	0.772	VW6	AXIAL-TENSION	As Requested	As Requested

AASHTO Truss LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

SE Truss (Upstream)

Analysis Results - SE Truss (Upstream)

Report Type: Rating Results Summary | Lane/Impact Loading Type: As Requested Detailed | Display Format: Single rating level per row

Live Load	Live Load Type	Rating Method	Rating Level	Load Rating (Ton)	Rating Factor	Element Name	Limit State	Impact	Lane
HS 20-44	Axle Load	LFD	Inventory	15.52	0.431	VW1	AXIAL-TENSION	As Requested	As Requested
HS 20-44	Axle Load	LFD	Operating	25.92	0.720	VW1	AXIAL-TENSION	As Requested	As Requested
Type 3	Axle Load	LFD	Inventory	13.20	0.528	VW1	AXIAL-TENSION	As Requested	As Requested
Type 3	Axle Load	LFD	Operating	22.05	0.882	VW1	AXIAL-TENSION	As Requested	As Requested
Type 3-3	Axle Load	LFD	Inventory	22.77	0.569	DW2	AXIAL-TENSION	As Requested	As Requested
Type 3-3	Axle Load	LFD	Operating	38.03	0.951	DW2	AXIAL-TENSION	As Requested	As Requested
Type 3S2	Axle Load	LFD	Inventory	19.63	0.545	VW1	AXIAL-TENSION	As Requested	As Requested
Type 3S2	Axle Load	LFD	Operating	32.78	0.911	VW1	AXIAL-TENSION	As Requested	As Requested
HS 20-44	Lane	LFD	Inventory	16.44	0.457	VW1	AXIAL-TENSION	As Requested	As Requested
HS 20-44	Lane	LFD	Operating	27.45	0.763	VW1	AXIAL-TENSION	As Requested	As Requested

AASHTO Truss LFR Engine Version 6.6.0.3001
Analysis Preference Setting: None

Close

TIMBER BRIDGE RATING AND DESIGN
FOR
AASHTO 16th Edition LOADS

MADERO
Version 1.02.05
Sep. 11, 2012

University of Wyoming
Laramie, Wyoming

ECHO INPUT FILE =====

COMMENT Agency: AASHTO
COMMENT Bridge Code: L15672000_02001
COMMENT Bridge Name: Swan River (Truss)
COMMENT Structure Definition: Timber Deck Model
COMMENT Superstructure for DECK RATING ONLY
TITLE Member Alt: Girder
UNITS 0
TYPE 1, 3, 4
CONTROL 1, 0, 2, 1, , , , 0, 1, 1, 1
OUTPUT 1, 0, 1, 0, 0, 1, 0, 0, 0, 0, 0, 1
COMMENT The option to generate points of interest at cross section
COMMENT changes was selected but is not supported by Madero.
REPORT 100
REPORT 101
REPORT 102
REPORT 103
REPORT 104
REPORT 105
REPORT 106
REPORT 107
REPORT 108
REPORT 109
REPORT 110

DeckHS20-44. OUT

CALCS	100
CALCS	101
CALCS	102
CALCS	103
CALCS	104
CALCS	105
CALCS	106
CALCS	107
CALCS	108
CALCS	109
CALCS	110
DECKMATL	DFL-WWPA, N1B
ADJUST	2, 2.000, 0.850, 0.970, 0.670, 0.900,
SPAN	17.0000
FIXITY	1, 10
FIXITY	2, 10
GENLDIM	15.5417, 1, 3.2500, 2.5000,
STRINGSPAC	1, 2.7500
STRINGSPAC	2, 2.0000
STRINGSPAC	3, 2.0000
STRINGSPAC	4, 2.0000
STRINGSPAC	5, 2.0000
STRINGSPAC	6, 2.0000
STRINGSPAC	7, 2.7500
STRINGDIM	8, 4.0010, , 0.2917
DECKDIM	4.0000, , , , 0, , 9.5000,
DECKSTR	WET, WET, , 1.150, 0.095, 0.625, 1800.000
STRINGDEAD	, 50.0000, 50.0000, 19.5000
COMMENT	The following commands contain wheel fractions.
EXBEAM	1, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010
EXREACT	1, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010

```

                                DeckHS20-44. OUT
EXREACT                2, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010
EFFWIDTH                20.0004, 10.0000
COMMENT                The beta live parameter on the COMBFACT command is used
COMMENT                to export the product of the Scale Factor and the Impact
COMMENT                Factor for each vehicle. See the Advanced button in the
COMMENT                Analysis Settings window.
COMMENT                -----
COMMENT                Within Madero, the magnitude of a lane load or pedestrian
COMMENT                load is fixed at 0.64 k/ft (9.3 N/mm). Therefore, the
COMMENT                IMPACT command is used to scale the Madero lane load so
COMMENT                it is equivalent to the load specified in BrDR.
COMMENT                -----
COMMENT                See the Custom I load group for Inventory rating results.
COMMENT                See the Custom II load group for Operating rating results.
COMMENT                Vehicle: HS 20-44
COMBFACT                4, 1.0, 1.000, 100.00, 0.5
COMBFACT                5, 1.0, 1.000, 133.00, 0.5
LIVE                   4, 1, 1.0000, 1, 0, 0, 0
LIVE                   5, 1, 1.0000, 1, 0, 0, 0

```

END INPUT FILE =====

CHECKING ROUTINE DETECTED ONE SPAN BRIDGE
AND THEREFORE THE VARIABLE AXLE SPACING
CONTROL WAS SET TO "NO". USE CONTROL COMMAND
TO OVERRIDE. SEE OUTPUT BELOW FOR CURRENT
CONTROL VARIABLES.

===== ADMINISTRATIVE INFORMATION =====

JOB TITLE: Member Alt: Girder

DATE : 12/04/2014
TIME : 9:18:35

♀ ===== SUMMARY OF CONTROL PARAMETERS =====

OUTPUT OPTIONS
Include Page Breaks : Yes
Table Output Abbreviated : No
Table Headers : Yes
Influence Function Report : No

DeckHS20-44. OUT

Detailed Action Report : No
 Load Calculations : Yes
 Adjustment Factor Calculations : No
 Rating Factor Calculations : No
 Stress Calculations : No
 Connection Calculations : No
 Structural Analysis Output Level : Minimum

UNITS: U. S. customary (kips, ft)

CONTROL PARAMETERS

Data Checking : Yes
 Variable Axle Spacing : No
 Neglect Axles That Do Not Contribute to Critical Load Effect : Yes
 One or Multiple Lanes Loaded : One
 Interior or Exterior Stringer : Exterior

LOAD COMBINATIONS

GROUP I LOAD GROUP
 Design Truck : Yes
 Truck Load Factor : 1.000
 Design Lane Load : Yes
 Special Truck #1 : No
 Special Truck #2 : No
 Special Truck #3 : No

CUSTOM I LOAD GROUP
 Design Truck : Yes
 Truck Load Factor : 1.000
 Design Lane Load : Yes
 Special Truck #1 : No
 Special Truck #2 : No
 Special Truck #3 : No

CUSTOM II LOAD GROUP
 Design Truck : Yes
 Truck Load Factor : 1.000
 Design Lane Load : Yes
 Special Truck #1 : No
 Special Truck #2 : No
 Special Truck #3 : No

♀

===== GENERAL STRUCTURE INFORMATION =====

Bridge Type: Steel Stringer with Timber Floor

```

=====
^           ^
Spans      1
Nodes      1           11
Elements   1 to 10
Supports   1           2
    
```

SPAN DATA

Span No. Length (ft)
 1 17.00

RESTRAINT DATA

Support No. Vertical -Rotation

DeckHS20-44. OUT

1 Fixed-Free
2 Fixed-Free

Roadway Width = 15.542 (ft)
Number of Design Lanes = 001
Curb Width = 3.250 (in)
Railing Width = 3.250 (in)
Wearing Surface Thickness = 2.500 (in)

NOMINAL DIMENSION IDENTIFICATION (per 1997 NDS Table 1A and AASHTO Table 13.2.2A)

Input Data:

Actual Width = 10.000 (in)
Actual Depth = 4.000 (in)
Section Type = Plank Deck

Output data:

Nominal Width = 10.000 (in)
Nominal Depth = 4.000 (in)

WHEEL LOAD DISTRIBUTION WIDTH CALCULATIONS (per AASHTO 16th ed. 3.25)

Input Data:

Number of Lanes = 1
Deck Type = Plank Deck
Deck Thickness = 4.000 (in)
Deck Member Width = 10.000 (in)
Stringer Spacing = 2.750 (ft)
Stringer Width = 4.001 (in)

Output data:

Effective Deck Span = 2.583 (ft)
Distribution Width Perpendicular to Deck Span = 10.000 (in)
Distribution Widths in Direction of Deck Span:
CUSTOM I Load Group HS Trk 20.000 (in)
CUSTOM II Load Group HS Trk 20.000 (in)

DEAD LOAD CALCULATIONS FOR TRANSVERSE DECK

Input data:

Stringer Spacing = 2.750 (ft)
Section Type = Plank Deck
Deck Thickness = 4.000 (in)
Wearing Surface Thickness = 2.500 (in)
Deck Unit Weight = 50.000 (pcf)
Wearing Surface Unit Weight = 50.000 (pcf)
Rail System Linear Weight = 19.500 (lb/ft)
WL Distribution Width Perpendicular to Deck Span = 10.000 (in)

Output data:

Deck Component DL = 0.014 (kip/ft)
Deck Wearing Surface DL = 0.009 (kip/ft)
Rail System DL = 0.020 (kip/ft)

GEOMETRIC SECTION PROPERTIES CALCULATIONS

Input Data:

Member Type = Plank Deck
Member Width = 10.000 (in)

DeckHS20-44. OUT
 Member Depth = 4.000 (in)
 Modulus of Elasticity in x = 0.180E+04 (ksi)
 Moisture Adjustment Factor = 0.900

Output data:

Moment of Inertia = 0.533E+02 (in⁴)
 Adjusted Modulus (E') = 0.162E+04 (ksi)
 Flexural Rigidity (E'I) = 0.864E+05 (k*in²)

♀ ===== SUMMARY OF TRANSVERSE DECK INFORMATION =====

MATERIAL DESIGNATIONS

Deck Type = Plank Deck
 Species = User
 Grade = User
 Grading Agency = User

STRENGTH PROPERTIES

Moisture Condition for Shear & Flexure = WET
 Moisture Condition for Bearing = WET
 Allowable Deflection = 0.040 (in)
 Allowable Bending Stress (tens. zone in tension) = 1.150 (ksi)
 Allowable Bending Stress (comp. zone in tension) = 1.150 (ksi)
 Allowable Shear Stress = 0.095 (ksi)
 Allowable Comp. Stress Perp to Grain (bot face) = 0.625 (ksi)
 Allowable Comp. Stress Perp to Grain (top face) = 0.625 (ksi)
 Modulus of Elasticity in x = 1800.000 (ksi)
 Modulus of Elasticity in y = 1800.000 (ksi)

GEOMETRIC PROPERTIES

Deck Overhang = 0.292 (ft)
 Actual Thickness = 4.000 (in)
 Effective Width = 10.000 (in)
 Moment of Inertia = 0.5333E+02 (in⁴)
 Flexural Rigidity = 0.8640E+05 (k*in²)

♀ TRANSVERSE DECK DETAILED REPORT

CUSTOM I LOAD GROUP

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Section Dimensions (in)

Depth 4.000 Effective Width 10.000

Design Stresses (ksi)

	Unfact. Stress	Moi st. Factor	Size Factor	Fl at Load Factor	Dur. Factor	Percent Fact. Factor	Fact. Stress
Flexure	1.150	0.85	1.10	1.10	1.15	1.00	1.564

Allowable Deflection (in) 0.040

Maximum Factored Moments (kip*ft)

	Moment	
	one lane	multiple lanes
Main Deck		
Component Dead	0.01	
Wearing Dead	0.01	
Live Load		
HS Trck	5.60	0.00

		DeckHS20-44. OUT			
Combi ned		5. 61		0. 00	
Cri ti cal		5. 61		0. 00	
Overhang					
Component Dead				0. 01	
Weari ng Dead				0. 00	
Li ve Load		one	mul ti ple		
		lane	lanes		
HS Trck		0. 00		0. 00	
Combi ned		0. 01		0. 00	
Cri ti cal		0. 01		0. 00	
Stresses		Fl exural (ksi)		Defl ecti on (i n)	
		-----		-----	
Main Deck					
Component Dead		0. 004		. 161E-03	
Weari ng Dead		0. 003		. 101E-03	
Li ve Load		one	mul ti ple'	one	mul ti ple
		lane	lanes '	lane	lanes
HS Trck		2. 520	0. 000	. 759E-01	. 000E+00
Combi ned		2. 527	0. 000		
Cri ti cal		2. 527	0. 000	. 759E-01	. 000E+00
Rati o		0. 62	0. 00	0. 53	0. 00
RF		0. 62	0. 00		
Overhang					
Component Dead		0. 003			
Weari ng Dead		0. 000			
Li ve Load		one	mul ti ple		
		lane	lanes		
HS Trck		0. 000	0. 000		
Combi ned		0. 003	0. 000		
Cri ti cal		0. 003	0. 000		
Rati o		491. 47	0. 00		
RF		*****	0. 00		
Cri ti cal Rati o		0. 62	*****	0. 53	*****
Cri ti cal RF		0. 62	*****		

♀

TRANSVERSE DECK DETAILED REPORT

CUSTOM II LOAD GROUP

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Section Dimensions (i n)

Depth 4. 000 Effective Width 10. 000

Design Stresses (ksi)

Unfact. Stress	Moi st. Factor	Size Factor	Fl at Factor	Load Dur. Factor	Percent Factor	Fact. Stress
1. 150	0. 85	1. 10	1. 10	1. 15	1. 33	2. 080

Allowable Defl ecti on (i n) 0. 040

Maximum Factored Moments (ki p*ft)

Moment

DeckHS20-44. OUT

Main Deck			
Component Dead		0.01	
Wearing Dead		0.01	
Live Load	one	mul ti pl e	
	lane	lanes	
HS Trck	5.60	0.00	
Combi ned	5.61	0.00	
Cri ti cal	5.61	0.00	

Overhang			
Component Dead		0.01	
Wearing Dead		0.00	
Live Load	one	mul ti pl e	
	lane	lanes	
HS Trck	0.00	0.00	
Combi ned	0.01	0.00	
Cri ti cal	0.01	0.00	

Stresses	Fl exural (ksi)	Defl ecti on (i n)
	-----	-----

Main Deck				
Component Dead		0.004		.161E-03
Wearing Dead		0.003		.101E-03
Live Load	one	mul ti pl e'	one	mul ti pl e
	lane	lanes'	lane	lanes
HS Trck	2.520	0.000	.759E-01	.000E+00
Combi ned	2.527	0.000		
Cri ti cal	2.527	0.000	.759E-01	.000E+00
Ratio	0.82	0.00	0.53	0.00
RF	0.82	0.00		

Overhang			
Component Dead		0.003	
Wearing Dead		0.000	
Live Load	one	mul ti pl e	
	lane	lanes	
HS Trck	0.000	0.000	
Combi ned	0.003	0.000	
Cri ti cal	0.003	0.000	
Ratio	653.66	0.00	
RF	*****	0.00	

Cri ti cal Ratio	0.82	*****	0.53	*****
Cri ti cal RF	0.82	*****		

♀

TRANSVERSE DECK SUMMARY REPORT

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Fl exural (ksi)	Defl ecti on (i n)
-----	-----
one	one
lane	lane
mul ti pl e	mul ti pl e
lanes	lanes
-----	-----

DeckHS20-44. OUT

CUSTOM I	LOAD GROUP				
Stress		2.527	0.000	0.759E-01	0.000E+00
Control		Main	Main	Main	Main
Truck		HS 14ft	HS 14ft	HS 14ft	HS 14ft
Allow		1.564	1.564	0.400E-01	0.400E-01
Ratio		0.62	*****	0.53	*****
RF		0.62	*****		
CUSTOM II	LOAD GROUP				
Stress		2.527	0.000	0.759E-01	0.000E+00
Control		Main	Main	Main	Main
Truck		HS 14ft	HS 14ft	HS 14ft	HS 14ft
Allow		2.080	2.080	0.400E-01	0.400E-01
Ratio		0.82	*****	0.53	*****
RF		0.82	*****		
Cr Ratio		0.62	*****	0.53	*****
Crit RF		0.62	*****		

Critical ratio for all limit states: 0.53
 Critical rating factor for all limit states: 0.62

TIMBER BRIDGE RATING AND DESIGN
FOR
AASHTO 16th Edition LOADS

MADERO
Version 1.02.05
Sep. 11, 2012

University of Wyoming
Laramie, Wyoming

ECHO INPUT FILE =====

COMMENT Agency: AASHTO
COMMENT Bridge Code: L15672000_02001
COMMENT Bridge Name: Swan River (Truss)
COMMENT Structure Definition: Timber Deck Model
COMMENT Superstructure for DECK RATING ONLY
TITLE Member Alt: Girder
UNITS 0
TYPE 1, 3, 4
CONTROL 1, 0, 2, 1, , , , 0, 1, 1, 1
OUTPUT 1, 0, 1, 0, 0, 1, 0, 0, 0, 0, 0, 1
COMMENT The option to generate points of interest at cross section
COMMENT changes was selected but is not supported by Madero.
REPORT 100
REPORT 101
REPORT 102
REPORT 103
REPORT 104
REPORT 105
REPORT 106
REPORT 107
REPORT 108
REPORT 109
REPORT 110

DeckType3. OUT

CALCS	100
CALCS	101
CALCS	102
CALCS	103
CALCS	104
CALCS	105
CALCS	106
CALCS	107
CALCS	108
CALCS	109
CALCS	110
DECKMATL	DFL-WWPA, N1B
ADJUST	2, 2.000, 0.850, 0.970, 0.670, 0.900,
SPAN	17.0000
FIXITY	1, 10
FIXITY	2, 10
GENLDIM	15.5417, 1, 3.2500, 2.5000,
STRINGSPAC	1, 2.7500
STRINGSPAC	2, 2.0000
STRINGSPAC	3, 2.0000
STRINGSPAC	4, 2.0000
STRINGSPAC	5, 2.0000
STRINGSPAC	6, 2.0000
STRINGSPAC	7, 2.7500
STRINGDIM	8, 4.0010, , 0.2917
DECKDIM	4.0000, , , , 0, , 9.5000,
DECKSTR	WET, WET, , 1.150, 0.095, 0.625, 1800.000
STRINGDEAD	, 50.0000, 50.0000, 19.5000
COMMENT	The following commands contain wheel fractions.
EXBEAM	1, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010
EXREACT	1, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010

```

DeckType3. OUT
EXREACT          2, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010
EFFWIDTH        14.5775, 10.0000
COMMENT          The beta live parameter on the COMBFACT command is used
COMMENT          to export the product of the Scale Factor and the Impact
COMMENT          Factor for each vehicle. See the Advanced button in the
COMMENT          Analysis Settings window.
COMMENT          -----
COMMENT          Within Madero, the magnitude of a lane load or pedestrian
COMMENT          load is fixed at 0.64 k/ft (9.3 N/mm). Therefore, the
COMMENT          IMPACT command is used to scale the Madero lane load so
COMMENT          it is equivalent to the load specified in BrDR.
COMMENT          -----
COMMENT          See the Custom I load group for Inventory rating results.
COMMENT          See the Custom II load group for Operating rating results.
COMMENT          Vehicle: Type 3
COMBFACT         4, 1.0, 1.000, 100.00, 0.5
COMBFACT         5, 1.0, 1.000, 133.00, 0.5
COMMENT          The axle loading is represented by Special Truck 1.
COMMENT          This only occurs when the axle loading does not conform
COMMENT          to the axle loads and spacings of an HS type truck.
TRUCK            16.000, 0.000 ,17.000, 15.000 ,17.000, 4.000
LIVE             4, 0, 1.0000, 0, 1, 0, 0
LIVE             5, 0, 1.0000, 0, 1, 0, 0

```

END INPUT FILE =====

CHECKING ROUTINE DETECTED ONE SPAN BRIDGE
AND THEREFORE THE VARIABLE AXLE SPACING
CONTROL WAS SET TO "NO". USE CONTROL COMMAND
TO OVERRIDE. SEE OUTPUT BELOW FOR CURRENT
CONTROL VARIABLES.

===== ADMINISTRATIVE INFORMATION =====

JOB TITLE: Member Alt: Girder

DATE : 12/04/2014
TIME : 9:18:35

♀ ===== SUMMARY OF CONTROL PARAMETERS =====

OUTPUT OPTIONS

Include Page Breaks : Yes
 Table Output Abbreviated : No
 Table Headers : Yes
 Influence Function Report : No
 Detailed Action Report : No
 Load Calculations : Yes
 Adjustment Factor Calculations : No
 Rating Factor Calculations : No
 Stress Calculations : No
 Connection Calculations : No
 Structural Analysis Output Level : Minimum

UNITS: U. S. customary (kips, ft)

CONTROL PARAMETERS

Data Checking : Yes
 Variable Axle Spacing : No
 Neglect Axles That Do Not Contribute
 to Critical Load Effect : Yes
 One or Multiple Lanes Loaded : One
 Interior or Exterior Stringer : Exterior

LOAD COMBINATIONS

GROUP I LOAD GROUP
 Design Truck : Yes
 Truck Load Factor : 1.000
 Design Lane Load : Yes
 Special Truck #1 : No
 Special Truck #2 : No
 Special Truck #3 : No

CUSTOM I LOAD GROUP
 Design Truck : No
 Truck Load Factor : 1.000
 Design Lane Load : No
 Special Truck #1 : Yes
 Special Truck #2 : No
 Special Truck #3 : No

CUSTOM II LOAD GROUP
 Design Truck : No
 Truck Load Factor : 1.000
 Design Lane Load : No
 Special Truck #1 : Yes
 Special Truck #2 : No
 Special Truck #3 : No

♀

===== GENERAL STRUCTURE INFORMATION =====

Bridge Type: Steel Stringer with Timber Floor

=====

	^		^
Spans		1	
Nodes	1		11
Elements		1 to 10	
Supports	1		2

DeckType3. OUT

SPAN DATA
 Span No. Length (ft)
 1 17.00

RESTRAINT DATA
 Support No. Vertical -Rotation
 1 Fixed-Free
 2 Fixed-Free

Roadway Width = 15.542 (ft)
 Number of Design Lanes = 001
 Curb Width = 3.250 (in)
 Railing Width = 3.250 (in)
 Wearing Surface Thickness = 2.500 (in)

NOMINAL DIMENSION IDENTIFICATION (per 1997 NDS Table 1A and AASHTO Table 13.2.2A)

Input Data:
 Actual Width = 10.000 (in)
 Actual Depth = 4.000 (in)
 Section Type = Plank Deck

Output data:
 Nominal Width = 10.000 (in)
 Nominal Depth = 4.000 (in)

WHEEL LOAD DISTRIBUTION WIDTH CALCULATIONS (per AASHTO 16th ed. 3.25)

Input Data:
 Number of Lanes = 1
 Deck Type = Plank Deck
 Deck Thickness = 4.000 (in)
 Deck Member Width = 10.000 (in)
 Stringer Spacing = 2.750 (ft)
 Stringer Width = 4.001 (in)

Output data:
 Effective Deck Span = 2.583 (ft)
 Distribution Width Perpendicular to Deck Span = 10.000 (in)
 Distribution Widths in Direction of Deck Span:
 CUSTOM I Load Group HS Trk 14.578 (in)
 Spec. 1 14.578 (in)
 CUSTOM II Load Group HS Trk 14.578 (in)
 Spec. 1 14.578 (in)

DEAD LOAD CALCULATIONS FOR TRANSVERSE DECK

Input data:
 Stringer Spacing = 2.750 (ft)
 Section Type = Plank Deck
 Deck Thickness = 4.000 (in)
 Wearing Surface Thickness = 2.500 (in)
 Deck Unit Weight = 50.000 (pcf)
 Wearing Surface Unit Weight = 50.000 (pcf)
 Rail System Linear Weight = 19.500 (lb/ft)
 WL Distribution Width Perpendicular to Deck Span = 10.000 (in)

Output data:

DeckType3. OUT
 Deck Component DL = 0.014 (kip/ft)
 Deck Wearing Surface DL = 0.009 (kip/ft)
 Rail System DL = 0.020 (kip/ft)

GEOMETRIC SECTION PROPERTIES CALCULATIONS

Input Data:

Member Type = Plank Deck
 Member Width = 10.000 (in)
 Member Depth = 4.000 (in)
 Modulus of Elasticity in x = 0.180E+04 (ksi)
 Moisture Adjustment Factor = 0.900

Output data:

Moment of Inertia = 0.533E+02 (in⁴)
 Adjusted Modulus (E') = 0.162E+04 (ksi)
 Flexural Rigidity (E'I) = 0.864E+05 (k*in²)

♀ ===== SUMMARY OF TRANSVERSE DECK INFORMATION =====

MATERIAL DESIGNATIONS

Deck Type = Plank Deck
 Species = User
 Grade = User
 Grading Agency = User

STRENGTH PROPERTIES

Moisture Condition for Shear & Flexure = WET
 Moisture Condition for Bearing = WET
 Allowable Deflection = 0.040 (in)
 Allowable Bending Stress (tens. zone in tension) = 1.150 (ksi)
 Allowable Bending Stress (comp. zone in tension) = 1.150 (ksi)
 Allowable Shear Stress = 0.095 (ksi)
 Allowable Comp. Stress Perp to Grain (bot face) = 0.625 (ksi)
 Allowable Comp. Stress Perp to Grain (top face) = 0.625 (ksi)
 Modulus of Elasticity in x = 1800.000 (ksi)
 Modulus of Elasticity in y = 1800.000 (ksi)

GEOMETRIC PROPERTIES

Deck Overhang = 0.292 (ft)
 Actual Thickness = 4.000 (in)
 Effective Width = 10.000 (in)
 Moment of Inertia = 0.5333E+02 (in⁴)
 Flexural Rigidity = 0.8640E+05 (k*in²)

♀ TRANSVERSE DECK DETAILED REPORT

CUSTOM I LOAD GROUP

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Section Dimensions (in)
 Depth 4.000 Effective Width 10.000

Design Stresses (ksi)	Unfact. Stress	Moi st. Factor	Size Factor	Fl at Load Dur. Factor	Percent Fact. Factor	Fact. Stress
Flexure	1.150	0.85	1.10	1.10	1.15	1.00 1.564

Allowable Deflection (in) 0.040

DeckType3. OUT

Maximum Factored Moments (kip*ft)

	Moment			
	one	multiple	one	multiple
	lane	lanes	lane	lanes
Main Deck				
Component Dead				
Wearing Dead				
Live Load				
Spec. 1	3.36	0.00		
Combined	3.37	0.00		
Critical	3.37	0.00		
Overhang				
Component Dead				
Wearing Dead				
Live Load				
Spec. 1	0.00	0.00		
Combined	0.01	0.00		
Critical	0.01	0.00		
Stresses				
	Flexural (ksi)		Deflection (in)	
Main Deck				
Component Dead				
Wearing Dead				
Live Load				
Spec. 1	1.512	0.000	.441E-01	.000E+00
Combined	1.518	0.000		
Critical	1.518	0.000	.441E-01	.000E+00
Ratio	1.03	0.00	0.91	0.00
RF	1.03	0.00		
Overhang				
Component Dead				
Wearing Dead				
Live Load				
Spec. 1	0.000	0.000		
Combined	0.003	0.000		
Critical	0.003	0.000		
Ratio	491.47	0.00		
RF	*****	0.00		
Critical Ratio	1.03	*****	0.91	*****
Critical RF	1.03	*****		

♀

TRANSVERSE DECK DETAILED REPORT

CUSTOM II LOAD GROUP

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Section Dimensions (in)
Depth 4.000

Effective Width 10.000
Page 7

DeckType3. OUT

Design Stresses (ksi)	Unfact. Stress	Moist. Factor	Size Factor	Flat Factor	Load Dur. Factor	Percent Factor	Fact. Stress
Flexure	1.150	0.85	1.10	1.10	1.15	1.33	2.080

Allowable Deflection (in) 0.040

Maximum Factored Moments (kip*ft)

	Moment	

Main Deck		
Component Dead	0.01	
Wearing Dead	0.01	
Live Load	one lane	multiple lanes
Spec. 1	3.36	0.00
Combined	3.37	0.00
Critical	3.37	0.00
Overhang		
Component Dead	0.01	
Wearing Dead	0.00	
Live Load	one lane	multiple lanes
Spec. 1	0.00	0.00
Combined	0.01	0.00
Critical	0.01	0.00

Stresses	Flexural (ksi)		Deflection (in)	
	-----		-----	
Main Deck				
Component Dead	0.004		.161E-03	
Wearing Dead	0.003		.101E-03	
Live Load	one lane	multiple lanes	one lane	multiple lanes
Spec. 1	1.512	0.000	.441E-01	.000E+00
Combined	1.518	0.000		
Critical	1.518	0.000	.441E-01	.000E+00
Ratio	1.37	0.00	0.91	0.00
RF	1.37	0.00		
Overhang				
Component Dead	0.003			
Wearing Dead	0.000			
Live Load	one lane	multiple lanes		
Spec. 1	0.000	0.000		
Combined	0.003	0.000		
Critical	0.003	0.000		
Ratio	653.66	0.00		
RF	*****	0.00		
Critical Ratio	1.37	*****	0.91	*****
Critical RF	1.37	*****		

DeckType3.OUT
TRANSVERSE DECK SUMMARY REPORT

♀

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

		Flexural (ksi)		Deflection (in)	
		one	mul ti pl e	one	mul ti pl e
		l ane	l anes	l ane	l anes
		-----	-----	-----	-----
CUSTOM I	LOAD GROUP				
Stress		1.518	0.000	0.441E-01	0.000E+00
Control		Main	Main	Main	Main
Truck		HS 30ft	HS 30ft	HS 30ft	HS 30ft
Allow		1.564	1.564	0.400E-01	0.400E-01
Ratio		1.03	*****	0.91	*****
RF		1.03	*****		
CUSTOM II	LOAD GROUP				
Stress		1.518	0.000	0.441E-01	0.000E+00
Control		Main	Main	Main	Main
Truck		HS 30ft	HS 30ft	HS 30ft	HS 30ft
Allow		2.080	2.080	0.400E-01	0.400E-01
Ratio		1.37	*****	0.91	*****
RF		1.37	*****		
Cr Ratio		1.03	*****	0.91	*****
Crit RF		1.03	*****		
Critical ratio for all limit states:				0.91	
Critical rating factor for all limit states:				1.03	

TIMBER BRIDGE RATING AND DESIGN
FOR
AASHTO 16th Edition LOADS

MADERO
Version 1.02.05
Sep. 11, 2012

University of Wyoming
Laramie, Wyoming

ECHO INPUT FILE =====

COMMENT Agency: AASHTO
COMMENT Bridge Code: L15672000_02001
COMMENT Bridge Name: Swan River (Truss)
COMMENT Structure Definition: Timber Deck Model
COMMENT Superstructure for DECK RATING ONLY
TITLE Member Alt: Girder
UNITS 0
TYPE 1, 3, 4
CONTROL 1, 0, 2, 1, , , , 0, 1, 1, 1
OUTPUT 1, 0, 1, 0, 0, 1, 0, 0, 0, 0, 0, 1
COMMENT The option to generate points of interest at cross section
COMMENT changes was selected but is not supported by Madero.
REPORT 100
REPORT 101
REPORT 102
REPORT 103
REPORT 104
REPORT 105
REPORT 106
REPORT 107
REPORT 108
REPORT 109
REPORT 110

DeckType3-3. OUT

CALCS	100
CALCS	101
CALCS	102
CALCS	103
CALCS	104
CALCS	105
CALCS	106
CALCS	107
CALCS	108
CALCS	109
CALCS	110
DECKMATL	DFL-WWPA, N1B
ADJUST	2, 2.000, 0.850, 0.970, 0.670, 0.900,
SPAN	17.0000
FIXITY	1, 10
FIXITY	2, 10
GENLDIM	15.5417, 1, 3.2500, 2.5000,
STRINGSPAC	1, 2.7500
STRINGSPAC	2, 2.0000
STRINGSPAC	3, 2.0000
STRINGSPAC	4, 2.0000
STRINGSPAC	5, 2.0000
STRINGSPAC	6, 2.0000
STRINGSPAC	7, 2.7500
STRINGDIM	8, 4.0010, , 0.2917
DECKDIM	4.0000, , , , 0, , 9.5000,
DECKSTR	WET, WET, , 1.150, 0.095, 0.625, 1800.000
STRINGDEAD	, 50.0000, 50.0000, 19.5000
COMMENT	The following commands contain wheel fractions.
EXBEAM	1, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010
EXREACT	1, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010

```

DeckType3-3. OUT
EXREACT          2, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010
EFFWIDTH        14.1422, 10.0000
COMMENT          The beta live parameter on the COMBFACT command is used
COMMENT          to export the product of the Scale Factor and the Impact
COMMENT          Factor for each vehicle. See the Advanced button in the
COMMENT          Analysis Settings window.
COMMENT          -----
COMMENT          Within Madero, the magnitude of a lane load or pedestrian
COMMENT          load is fixed at 0.64 k/ft (9.3 N/mm). Therefore, the
COMMENT          IMPACT command is used to scale the Madero lane load so
COMMENT          it is equivalent to the load specified in BrDR.
COMMENT          -----
COMMENT          See the Custom I load group for Inventory rating results.
COMMENT          See the Custom II load group for Operating rating results.
COMMENT          Vehicle: Type 3-3
COMBFACT         4, 1.0, 1.000, 100.00, 0.5
COMBFACT         5, 1.0, 1.000, 133.00, 0.5
COMMENT          The axle loading is represented by Special Truck 1.
COMMENT          This only occurs when the axle loading does not conform
COMMENT          to the axle loads and spacings of an HS type truck.
TRUCK           12.000, 0.000 ,12.000, 15.000 ,12.000, 4.000,
LIVE            4, 0, 1.0000, 0, 1, 0, 0
LIVE            5, 0, 1.0000, 0, 1, 0, 0

```

END INPUT FILE =====

CHECKING ROUTINE DETECTED ONE SPAN BRIDGE
AND THEREFORE THE VARIABLE AXLE SPACING
CONTROL WAS SET TO "NO". USE CONTROL COMMAND
TO OVERRIDE. SEE OUTPUT BELOW FOR CURRENT
CONTROL VARIABLES.

===== ADMINISTRATIVE INFORMATION =====

JOB TITLE: Member Alt: Girder

DATE : 12/04/2014
TIME : 9:18:35

DeckType3-3. OUT

♀ ===== SUMMARY OF CONTROL PARAMETERS =====

OUTPUT OPTIONS

Include Page Breaks : Yes
 Table Output Abbreviated : No
 Table Headers : Yes
 Influence Function Report : No
 Detailed Action Report : No
 Load Calculations : Yes
 Adjustment Factor Calculations : No
 Rating Factor Calculations : No
 Stress Calculations : No
 Connection Calculations : No
 Structural Analysis Output Level : Minimum

UNITS: U. S. customary (kips, ft)

CONTROL PARAMETERS

Data Checking : Yes
 Variable Axle Spacing : No
 Neglect Axles That Do Not Contribute
 to Critical Load Effect : Yes
 One or Multiple Lanes Loaded : One
 Interior or Exterior Stringer : Exterior

LOAD COMBINATIONS

GROUP I LOAD GROUP
 Design Truck : Yes
 Truck Load Factor : 1.000
 Design Lane Load : Yes
 Special Truck #1 : No
 Special Truck #2 : No
 Special Truck #3 : No

CUSTOM I LOAD GROUP
 Design Truck : No
 Truck Load Factor : 1.000
 Design Lane Load : No
 Special Truck #1 : Yes
 Special Truck #2 : No
 Special Truck #3 : No

CUSTOM II LOAD GROUP
 Design Truck : No
 Truck Load Factor : 1.000
 Design Lane Load : No
 Special Truck #1 : Yes
 Special Truck #2 : No
 Special Truck #3 : No

♀

===== GENERAL STRUCTURE INFORMATION =====

Bridge Type: Steel Stringer with Timber Floor

=====

	^		^
Spans		1	
Nodes	1		11
Elements		1 to 10	
Supports	1		2

DeckType3-3.0UT

SPAN DATA
 Span No. Length (ft)
 1 17.00

RESTRAINT DATA
 Support No. Vertical -Rotation
 1 Fixed-Free
 2 Fixed-Free

Roadway Width = 15.542 (ft)
 Number of Design Lanes = 001
 Curb Width = 3.250 (in)
 Railing Width = 3.250 (in)
 Wearing Surface Thickness = 2.500 (in)

NOMINAL DIMENSION IDENTIFICATION (per 1997 NDS Table 1A and AASHTO Table 13.2.2A)

Input Data:
 Actual Width = 10.000 (in)
 Actual Depth = 4.000 (in)
 Section Type = Plank Deck

Output data:
 Nominal Width = 10.000 (in)
 Nominal Depth = 4.000 (in)

WHEEL LOAD DISTRIBUTION WIDTH CALCULATIONS (per AASHTO 16th ed. 3.25)

Input Data:
 Number of Lanes = 1
 Deck Type = Plank Deck
 Deck Thickness = 4.000 (in)
 Deck Member Width = 10.000 (in)
 Stringer Spacing = 2.750 (ft)
 Stringer Width = 4.001 (in)

Output data:
 Effective Deck Span = 2.583 (ft)
 Distribution Width Perpendicular to Deck Span = 10.000 (in)
 Distribution Widths in Direction of Deck Span:
 CUSTOM I Load Group HS Trk 14.142 (in)
 Spec. 1 14.142 (in)
 CUSTOM II Load Group HS Trk 14.142 (in)
 Spec. 1 14.142 (in)

DEAD LOAD CALCULATIONS FOR TRANSVERSE DECK

Input data:
 Stringer Spacing = 2.750 (ft)
 Section Type = Plank Deck
 Deck Thickness = 4.000 (in)
 Wearing Surface Thickness = 2.500 (in)
 Deck Unit Weight = 50.000 (pcf)
 Wearing Surface Unit Weight = 50.000 (pcf)
 Rail System Linear Weight = 19.500 (lb/ft)
 WL Distribution Width Perpendicular to Deck Span = 10.000 (in)

Output data:

DeckType3-3. OUT
 Deck Component DL = 0.014 (kip/ft)
 Deck Wearing Surface DL = 0.009 (kip/ft)
 Rail System DL = 0.020 (kip/ft)

GEOMETRIC SECTION PROPERTIES CALCULATIONS

Input Data:

Member Type = Plank Deck
 Member Width = 10.000 (in)
 Member Depth = 4.000 (in)
 Modulus of Elasticity in x = 0.180E+04 (ksi)
 Moisture Adjustment Factor = 0.900

Output data:

Moment of Inertia = 0.533E+02 (in⁴)
 Adjusted Modulus (E') = 0.162E+04 (ksi)
 Flexural Rigidity (E'I) = 0.864E+05 (k*in²)

♀ ===== SUMMARY OF TRANSVERSE DECK INFORMATION =====

MATERIAL DESIGNATIONS

Deck Type = Plank Deck
 Species = User
 Grade = User
 Grading Agency = User

STRENGTH PROPERTIES

Moisture Condition for Shear & Flexure = WET
 Moisture Condition for Bearing = WET
 Allowable Deflection = 0.040 (in)
 Allowable Bending Stress (tens. zone in tension) = 1.150 (ksi)
 Allowable Bending Stress (comp. zone in tension) = 1.150 (ksi)
 Allowable Shear Stress = 0.095 (ksi)
 Allowable Comp. Stress Perp to Grain (bot face) = 0.625 (ksi)
 Allowable Comp. Stress Perp to Grain (top face) = 0.625 (ksi)
 Modulus of Elasticity in x = 1800.000 (ksi)
 Modulus of Elasticity in y = 1800.000 (ksi)

GEOMETRIC PROPERTIES

Deck Overhang = 0.292 (ft)
 Actual Thickness = 4.000 (in)
 Effective Width = 10.000 (in)
 Moment of Inertia = 0.5333E+02 (in⁴)
 Flexural Rigidity = 0.8640E+05 (k*in²)

♀ TRANSVERSE DECK DETAILED REPORT

CUSTOM I LOAD GROUP

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Section Dimensions (in)
 Depth 4.000 Effective Width 10.000

Design Stresses (ksi)	Unfact. Stress	Moi st. Factor	Size Factor	Flat Load Factor	Dur. Factor	Percent Fact. Stress
Flexure	1.150	0.85	1.10	1.10	1.15	1.00 1.564

Allowable Deflection (in) 0.040

DeckType3-3.0UT

Maximum Factored Moments (kip*ft)

	Moment		Deflection (in)	
	one lane	multiple lanes	one lane	multiple lanes
Main Deck				
Component Dead	0.01		.161E-03	
Wearing Dead	0.01		.101E-03	
Live Load				
Spec. 1	3.19	0.00	.417E-01	.000E+00
Combined	3.21	0.00	.417E-01	.000E+00
Critical	3.21	0.00	.417E-01	.000E+00
Ratio	1.08	0.00	0.96	0.00
RF	1.08	0.00		
Overhang				
Component Dead	0.01			
Wearing Dead	0.00			
Live Load				
Spec. 1	0.00	0.00		
Combined	0.01	0.00		
Critical	0.01	0.00		
Ratio	491.47	0.00		
RF	*****	0.00		
Critical Ratio	1.08	*****	0.96	*****
Critical RF	1.08	*****		

♀

TRANSVERSE DECK DETAILED REPORT

CUSTOM II LOAD GROUP

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Section Dimensions (in)
Depth 4.000

Effective Width 10.000
Page 7

DeckType3-3. OUT

Design Stresses (ksi)	Unfact. Stress	Moist. Factor	Size Factor	Flat Factor	Load Dur. Factor	Percent Factor	Fact. Stress
Flexure	1.150	0.85	1.10	1.10	1.15	1.33	2.080

Allowable Deflection (in) 0.040

Maximum Factored Moments (kip*ft)

	Moment	

Main Deck		
Component Dead	0.01	
Wearing Dead	0.01	
Live Load	one lane	multiple lanes
Spec. 1	3.19	0.00
Combined	3.21	0.00
Critical	3.21	0.00
Overhang		
Component Dead	0.01	
Wearing Dead	0.00	
Live Load	one lane	multiple lanes
Spec. 1	0.00	0.00
Combined	0.01	0.00
Critical	0.01	0.00

Stresses	Flexural (ksi)		Deflection (in)	
	-----		-----	
Main Deck				
Component Dead	0.004		.161E-03	
Wearing Dead	0.003		.101E-03	
Live Load	one lane	multiple lanes	one lane	multiple lanes
Spec. 1	1.436	0.000	.417E-01	.000E+00
Combined	1.442	0.000		
Critical	1.442	0.000	.417E-01	.000E+00
Ratio	1.44	0.00	0.96	0.00
RF	1.44	0.00		
Overhang				
Component Dead	0.003			
Wearing Dead	0.000			
Live Load	one lane	multiple lanes		
Spec. 1	0.000	0.000		
Combined	0.003	0.000		
Critical	0.003	0.000		
Ratio	653.66	0.00		
RF	*****	0.00		
Critical Ratio	1.44	*****	0.96	*****
Critical RF	1.44	*****		

DeckType3-3. OUT
TRANSVERSE DECK SUMMARY REPORT

♀

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

		Flexural (ksi)		Deflection (in)	
		one lane	mul ti pl e lanes	one lane	mul ti pl e lanes
		-----	-----	-----	-----
CUSTOM I	LOAD GROUP				
Stress		1.442	0.000	0.417E-01	0.000E+00
Control		Main	Main	Main	Main
Truck		HS 30ft	HS 30ft	HS 30ft	HS 30ft
Allow		1.564	1.564	0.400E-01	0.400E-01
Ratio		1.08	*****	0.96	*****
RF		1.08	*****		
CUSTOM II	LOAD GROUP				
Stress		1.442	0.000	0.417E-01	0.000E+00
Control		Main	Main	Main	Main
Truck		HS 30ft	HS 30ft	HS 30ft	HS 30ft
Allow		2.080	2.080	0.400E-01	0.400E-01
Ratio		1.44	*****	0.96	*****
RF		1.44	*****		
Cr Ratio		1.08	*****	0.96	*****
Crit RF		1.08	*****		
Critical ratio for all limit states:				0.96	
Critical rating factor for all limit states:				1.08	

TIMBER BRIDGE RATING AND DESIGN
FOR
AASHTO 16th Edition LOADS

MADERO
Version 1.02.05
Sep. 11, 2012

University of Wyoming
Laramie, Wyoming

ECHO INPUT FILE =====

COMMENT Agency: AASHTO
COMMENT Bridge Code: L15672000_02001
COMMENT Bridge Name: Swan River (Truss)
COMMENT Structure Definition: Timber Deck Model
COMMENT Superstructure for DECK RATING ONLY
TITLE Member Alt: Girder
UNITS 0
TYPE 1, 3, 4
CONTROL 1, 0, 2, 1, , , , 0, 1, 1, 1
OUTPUT 1, 0, 1, 0, 0, 1, 0, 0, 0, 0, 0, 1
COMMENT The option to generate points of interest at cross section
COMMENT changes was selected but is not supported by Madero.
REPORT 100
REPORT 101
REPORT 102
REPORT 103
REPORT 104
REPORT 105
REPORT 106
REPORT 107
REPORT 108
REPORT 109
REPORT 110

DeckType3S2. OUT

CALCS	100
CALCS	101
CALCS	102
CALCS	103
CALCS	104
CALCS	105
CALCS	106
CALCS	107
CALCS	108
CALCS	109
CALCS	110
DECKMATL	DFL-WWPA, N1B
ADJUST	2, 2.000, 0.850, 0.970, 0.670, 0.900,
SPAN	17.0000
FIXITY	1, 10
FIXITY	2, 10
GENLDIM	15.5417, 1, 3.2500, 2.5000,
STRINGSPAC	1, 2.7500
STRINGSPAC	2, 2.0000
STRINGSPAC	3, 2.0000
STRINGSPAC	4, 2.0000
STRINGSPAC	5, 2.0000
STRINGSPAC	6, 2.0000
STRINGSPAC	7, 2.7500
STRINGDIM	8, 4.0010, , 0.2917
DECKDIM	4.0000, , , , 0, , 9.5000,
DECKSTR	WET, WET, , 1.150, 0.095, 0.625, 1800.000
STRINGDEAD	, 50.0000, 50.0000, 19.5000
COMMENT	The following commands contain wheel fractions.
EXBEAM	1, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010
EXREACT	1, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010

```

                                DeckType3S2.OUT
EXREACT                2, 0.010, 0.010, 0.010, 0.010, 0.010, 0.010
EFFWIDTH                13.9195, 10.0000
COMMENT                 The beta live parameter on the COMBFACT command is used
COMMENT                 to export the product of the Scale Factor and the Impact
COMMENT                 Factor for each vehicle. See the Advanced button in the
COMMENT                 Analysis Settings window.
COMMENT                 -----
COMMENT                 Within Madero, the magnitude of a lane load or pedestrian
COMMENT                 load is fixed at 0.64 k/ft (9.3 N/mm). Therefore, the
COMMENT                 IMPACT command is used to scale the Madero lane load so
COMMENT                 it is equivalent to the load specified in BrDR.
COMMENT                 -----
COMMENT                 See the Custom I load group for Inventory rating results.
COMMENT                 See the Custom II load group for Operating rating results.
COMMENT                 Vehicle: Type 3S2
COMBFACT                4, 1.0, 1.000, 100.00, 0.5
COMBFACT                5, 1.0, 1.000, 133.00, 0.5
COMMENT                 The axle loading is represented by Special Truck 1.
COMMENT                 This only occurs when the axle loading does not conform
COMMENT                 to the axle loads and spacings of an HS type truck.
TRUCK                  10.000, 0.000 ,15.500, 11.000 ,15.500, 4.000,
LIVE                   4, 0, 1.0000, 0, 1, 0, 0
LIVE                   5, 0, 1.0000, 0, 1, 0, 0

```

END INPUT FILE =====

CHECKING ROUTINE DETECTED ONE SPAN BRIDGE
AND THEREFORE THE VARIABLE AXLE SPACING
CONTROL WAS SET TO "NO". USE CONTROL COMMAND
TO OVERRIDE. SEE OUTPUT BELOW FOR CURRENT
CONTROL VARIABLES.

===== ADMINISTRATIVE INFORMATION =====

JOB TITLE: Member Alt: Girder

DATE : 12/04/2014
TIME : 9:18:35

♀ ===== SUMMARY OF CONTROL PARAMETERS =====

OUTPUT OPTIONS

Include Page Breaks : Yes
 Table Output Abbreviated : No
 Table Headers : Yes
 Influence Function Report : No
 Detailed Action Report : No
 Load Calculations : Yes
 Adjustment Factor Calculations : No
 Rating Factor Calculations : No
 Stress Calculations : No
 Connection Calculations : No
 Structural Analysis Output Level : Minimum

UNITS: U. S. customary (kips, ft)

CONTROL PARAMETERS

Data Checking : Yes
 Variable Axle Spacing : No
 Neglect Axles That Do Not Contribute
 to Critical Load Effect : Yes
 One or Multiple Lanes Loaded : One
 Interior or Exterior Stringer : Exterior

LOAD COMBINATIONS

GROUP I LOAD GROUP
 Design Truck : Yes
 Truck Load Factor : 1.000
 Design Lane Load : Yes
 Special Truck #1 : No
 Special Truck #2 : No
 Special Truck #3 : No

CUSTOM I LOAD GROUP
 Design Truck : No
 Truck Load Factor : 1.000
 Design Lane Load : No
 Special Truck #1 : Yes
 Special Truck #2 : No
 Special Truck #3 : No

CUSTOM II LOAD GROUP
 Design Truck : No
 Truck Load Factor : 1.000
 Design Lane Load : No
 Special Truck #1 : Yes
 Special Truck #2 : No
 Special Truck #3 : No

♀

===== GENERAL STRUCTURE INFORMATION =====

Bridge Type: Steel Stringer with Timber Floor

=====
 ^ ^
 Spans 1
 Nodes 1 11
 Elements 1 to 10
 Supports 1 2

DeckType3S2. OUT

SPAN DATA

Span No. Length (ft)
1 17.00

RESTRAINT DATA

Support No. Vertical -Rotation
1 Fixed-Free
2 Fixed-Free

Roadway Width = 15.542 (ft)
Number of Design Lanes = 001
Curb Width = 3.250 (in)
Railing Width = 3.250 (in)
Wearing Surface Thickness = 2.500 (in)

NOMINAL DIMENSION IDENTIFICATION (per 1997 NDS Table 1A and AASHTO Table 13.2.2A)

Input Data:

Actual Width = 10.000 (in)
Actual Depth = 4.000 (in)
Section Type = Plank Deck

Output data:

Nominal Width = 10.000 (in)
Nominal Depth = 4.000 (in)

WHEEL LOAD DISTRIBUTION WIDTH CALCULATIONS (per AASHTO 16th ed. 3.25)

Input Data:

Number of Lanes = 1
Deck Type = Plank Deck
Deck Thickness = 4.000 (in)
Deck Member Width = 10.000 (in)
Stringer Spacing = 2.750 (ft)
Stringer Width = 4.001 (in)

Output data:

Effective Deck Span = 2.583 (ft)
Distribution Width Perpendicular to Deck Span = 10.000 (in)
Distribution Widths in Direction of Deck Span:
CUSTOM I Load Group HS Trk 13.920 (in)
Spec. 1 13.920 (in)
CUSTOM II Load Group HS Trk 13.920 (in)
Spec. 1 13.920 (in)

DEAD LOAD CALCULATIONS FOR TRANSVERSE DECK

Input data:

Stringer Spacing = 2.750 (ft)
Section Type = Plank Deck
Deck Thickness = 4.000 (in)
Wearing Surface Thickness = 2.500 (in)
Deck Unit Weight = 50.000 (pcf)
Wearing Surface Unit Weight = 50.000 (pcf)
Rail System Linear Weight = 19.500 (lb/ft)
WL Distribution Width Perpendicular to Deck Span = 10.000 (in)

Output data:

DeckType3S2. OUT
 Deck Component DL = 0.014 (kip/ft)
 Deck Wearing Surface DL = 0.009 (kip/ft)
 Rail System DL = 0.020 (kip/ft)

GEOMETRIC SECTION PROPERTIES CALCULATIONS

Input Data:

Member Type = Plank Deck
 Member Width = 10.000 (in)
 Member Depth = 4.000 (in)
 Modulus of Elasticity in x = 0.180E+04 (ksi)
 Moisture Adjustment Factor = 0.900

Output data:

Moment of Inertia = 0.533E+02 (in⁴)
 Adjusted Modulus (E') = 0.162E+04 (ksi)
 Flexural Rigidity (E'I) = 0.864E+05 (k*in²)

♀ ===== SUMMARY OF TRANSVERSE DECK INFORMATION =====

MATERIAL DESIGNATIONS

Deck Type = Plank Deck
 Species = User
 Grade = User
 Grading Agency = User

STRENGTH PROPERTIES

Moisture Condition for Shear & Flexure = WET
 Moisture Condition for Bearing = WET
 Allowable Deflection = 0.040 (in)
 Allowable Bending Stress (tens. zone in tension) = 1.150 (ksi)
 Allowable Bending Stress (comp. zone in tension) = 1.150 (ksi)
 Allowable Shear Stress = 0.095 (ksi)
 Allowable Comp. Stress Perp to Grain (bot face) = 0.625 (ksi)
 Allowable Comp. Stress Perp to Grain (top face) = 0.625 (ksi)
 Modulus of Elasticity in x = 1800.000 (ksi)
 Modulus of Elasticity in y = 1800.000 (ksi)

GEOMETRIC PROPERTIES

Deck Overhang = 0.292 (ft)
 Actual Thickness = 4.000 (in)
 Effective Width = 10.000 (in)
 Moment of Inertia = 0.5333E+02 (in⁴)
 Flexural Rigidity = 0.8640E+05 (k*in²)

♀ TRANSVERSE DECK DETAILED REPORT

CUSTOM I LOAD GROUP

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Section Dimensions (in)
 Depth 4.000 Effective Width 10.000

Design Stresses (ksi)	Unfact. Stress	Moi st. Factor	Size Factor	Fl at Load Dur. Factor	Percent Fact. Factor	Fact. Stress
Flexure	1.150	0.85	1.10	1.10	1.15	1.00 1.564

Allowable Deflection (in) 0.040

DeckType3S2. OUT

Maximum Factored Moments (kip*ft)

		Moment			
		-----		-----	
Main Deck					
Component Dead		0.01			
Wearing Dead		0.01			
Live Load					
		one	multiple		
		lane	lanes		
Spec. 1		3.11	0.00		
Combined		3.12	0.00		
Critical		3.12	0.00		
Overhang					
Component Dead		0.01			
Wearing Dead		0.00			
Live Load					
		one	multiple		
		lane	lanes		
Spec. 1		0.00	0.00		
Combined		0.01	0.00		
Critical		0.01	0.00		
Stresses		Flexural (ksi)		Deflection (in)	
		-----		-----	
Main Deck					
Component Dead		0.004		.161E-03	
Wearing Dead		0.003		.101E-03	
Live Load					
		one	multiple	one	multiple
		lane	lanes	lane	lanes
Spec. 1		1.397	0.000	.405E-01	.000E+00
Combined		1.404	0.000		
Critical		1.404	0.000	.405E-01	.000E+00
Ratio		1.11	0.00	0.99	0.00
RF		1.11	0.00		
Overhang					
Component Dead		0.003			
Wearing Dead		0.000			
Live Load					
		one	multiple		
		lane	lanes		
Spec. 1		0.000	0.000		
Combined		0.003	0.000		
Critical		0.003	0.000		
Ratio		491.47	0.00		
RF		*****	0.00		
Critical Ratio		1.11	*****	0.99	*****
Critical RF		1.11	*****		

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TRANSVERSE DECK DETAILED REPORT

CUSTOM II LOAD GROUP

NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

Section Dimensions (in)
Depth 4.000

Effective Width 10.000
Page 7

DeckType3S2. OUT

Design Stresses (ksi)	Unfact. Stress	Moi st. Factor	Size Factor	Fl at Factor	Load Dur. Factor	Percent Factor	Fact. Stress
Flexure	1.150	0.85	1.10	1.10	1.15	1.33	2.080

Allowable Deflection (in) 0.040

Maximum Factored Moments (kip*ft)

	Moment	

Main Deck		
Component Dead	0.01	
Wearing Dead	0.01	
Live Load	one lane	multiple lanes
Spec. 1	3.11	0.00
Combi ned	3.12	0.00
Cri ti cal	3.12	0.00
Overhang		
Component Dead	0.01	
Wearing Dead	0.00	
Live Load	one lane	multiple lanes
Spec. 1	0.00	0.00
Combi ned	0.01	0.00
Cri ti cal	0.01	0.00

Stresses	Flexural (ksi)		Deflection (in)	
	-----		-----	
Main Deck				
Component Dead	0.004		.161E-03	
Wearing Dead	0.003		.101E-03	
Live Load	one lane	multiple lanes	one lane	multiple lanes
Spec. 1	1.397	0.000	.405E-01	.000E+00
Combi ned	1.404	0.000		
Cri ti cal	1.404	0.000	.405E-01	.000E+00
Ratio	1.48	0.00	0.99	0.00
RF	1.48	0.00		
Overhang				
Component Dead	0.003			
Wearing Dead	0.000			
Live Load	one lane	multiple lanes		
Spec. 1	0.000	0.000		
Combi ned	0.003	0.000		
Cri ti cal	0.003	0.000		
Ratio	653.66	0.00		
RF	*****	0.00		
Cri ti cal Ratio	1.48	*****	0.99	*****
Cri ti cal RF	1.48	*****		

DeckType3S2.OUT
TRANSVERSE DECK SUMMARY REPORT

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NOTE: Asterisks indicate large ratios which are non-critical and may be ignored

		Flexural (ksi)		Deflection (in)	
		one lane	mul ti pl e lanes	one lane	mul ti pl e lanes
		-----	-----	-----	-----
CUSTOM I	LOAD GROUP				
Stress		1.404	0.000	0.405E-01	0.000E+00
Control		Main	Main	Main	Main
Truck		HS 30ft	HS 30ft	HS 30ft	HS 30ft
Allow		1.564	1.564	0.400E-01	0.400E-01
Ratio		1.11	*****	0.99	*****
RF		1.11	*****		
CUSTOM II	LOAD GROUP				
Stress		1.404	0.000	0.405E-01	0.000E+00
Control		Main	Main	Main	Main
Truck		HS 30ft	HS 30ft	HS 30ft	HS 30ft
Allow		2.080	2.080	0.400E-01	0.400E-01
Ratio		1.48	*****	0.99	*****
RF		1.48	*****		
Cr Ratio		1.11	*****	0.99	*****
Crit RF		1.11	*****		
Critical ratio for all limit states:				0.99	
Critical rating factor for all limit states:				1.11	

Bridge ID :L1567200002001
 Bridge : Swan River (Truss)
 StructDef : 119'SteelTruss(withReducedCapacityMembers)
 User : BrR
 Date : Thursday, December 04, 2014 09:06:35
 File : RatingResults.XML
 Analysis Preference Setting : None

NBI Structure ID :L15672000_02001
 Bridge Alt : As-built
 Member : NW Truss (Downstream)

Load Factor Rating Summary

Live Load	Live Load Type	Inv Element	Inv RF	Inv Capacity (Ton)	Opr Element	Opr RF	Opr Capacity (Ton)	Legal Opr Element	Legal Opr RF	Legal Opr Capacity (Ton)	Permit Inv Element	Permit Inv RF	Permit Inv Capacity (Ton)	Permit Opr Element	Permit Opr RF	Permit Opr Capacity (Ton)	Impact	Lane
HS 20-44	Design Truck	VW6	0.436	15.71	VW6	0.729	26.23										As Requested	As Requested
HS 20-44	Design Lane	VW1	0.462	16.63	VW6	0.772	27.78										As Requested	As Requested
HS 20-44	Design Truck	VW6	0.436	15.71	VW6	0.729	26.23										With Impact	Single Lane
HS 20-44	Design Lane	VW1	0.462	16.63	VW6	0.772	27.78										With Impact	Single Lane
HS 20-44	Design Truck	VW6	0.436	15.71	VW6	0.729	26.23										With Impact	Multi-Lane
HS 20-44	Design Lane	VW1	0.462	16.63	VW6	0.772	27.78										With Impact	Multi-Lane
HS 20-44	Design Truck	VW6	0.567	20.42	VW6	0.947	34.10										Without Impact	Single Lane
HS 20-44	Design Lane	VW1	0.601	21.63	VW6	1.003	36.11										Without Impact	Single Lane
HS 20-44	Design Truck	VW6	0.567	20.42	VW6	0.947	34.10										Without Impact	Multi-Lane
HS 20-44	Design Lane	VW1	0.601	21.63	VW6	1.003	36.11										Without Impact	Multi-Lane
Type 3	Design Truck	VW6	0.535	13.36	VW6	0.893	22.32										As Requested	As Requested
Type 3	Design Truck	VW6	0.535	13.36	VW6	0.893	22.32										With Impact	Single Lane
Type 3	Design Truck	VW6	0.535	13.36	VW6	0.893	22.32										With Impact	Multi-Lane
Type 3	Design Truck	VW6	0.695	17.37	VW6	1.160	29.01										Without Impact	Single Lane
Type 3	Design Truck	VW6	0.695	17.37	VW6	1.160	29.01										Without Impact	Multi-Lane
Type 3-3	Design Truck	DW5	0.577	23.07	DW5	0.963	38.52										As Requested	As Requested
Type 3-3	Design Truck	DW5	0.577	23.07	DW5	0.963	38.52										With Impact	Single Lane
Type 3-3	Design Truck	DW5	0.577	23.07	DW5	0.963	38.52										With Impact	Multi-Lane
Type 3-3	Design Truck	DW5	0.718	28.71	DW5	1.199	47.95										Without Impact	Single Lane
Type 3-3	Design Truck	DW5	0.718	28.71	DW5	1.199	47.95										Without Impact	Multi-Lane
Type 3S2	Design Truck	VW6	0.552	19.87	VW6	0.922	33.18										As Requested	As Requested
Type 3S2	Design Truck	VW6	0.552	19.87	VW6	0.922	33.18										With Impact	Single Lane
Type 3S2	Design Truck	VW6	0.552	19.87	VW6	0.922	33.18										With Impact	Multi-Lane
Type 3S2	Design Truck	DW5	0.712	25.64	DW5	1.189	42.82										Without Impact	Single Lane
Type 3S2	Design Truck	DW5	0.712	25.64	DW5	1.189	42.82										Without Impact	Multi-Lane

Detailed Truss Member Rating Results

Live Load: HS 20-44 (Design Truck)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand		Legal Opr RF	Permit Inv RF	Permit Opr RF			
			Comp. Inv / Opr / Legal Permit Inv / Opr / Permit Opr (kip)		Tens. Inv / Opr / Legal Permit Inv / Opr / Permit Opr (kip)		IF	Comp. (kip)	Tens. (kip)	Comp. (kip)	Tens. (kip)	One Lane LLDF				Multi Lane LLDF		
			IF	IF	IF	IF											Inv RF	Opr RF
BC1	Lower-Chord	20.61	////		45.39 / 45.39	////	1.20		88.02			0.665		0.775	1.294			
		20.61	////		45.39 / 45.39	////	1.20		88.02			0.665		0.775	1.294			
		20.61	////		45.39 / 45.39	////			88.02			0.665		0.934	1.559			
		20.61	////		45.39 / 45.39	////			88.02			0.665		0.934	1.559			
BC2	Lower-Chord	20.61	////		45.39 / 45.39	////	1.20		93.75			0.665		0.848	1.415			
		20.61	////		45.39 / 45.39	////	1.20		93.75			0.665		0.848	1.415			
		20.61	////		45.39 / 45.39	////			93.75			0.665		1.021	1.705			
		20.61	////		45.39 / 45.39	////			93.75			0.665		1.021	1.705			
BC3	Lower-Chord	34.46	////		74.12 / 74.12	////	1.20		135.00			0.665		0.699	1.168			
		34.46	////		74.12 / 74.12	////	1.20		135.00			0.665		0.699	1.168			
		34.46	////		74.12 / 74.12	////			135.00			0.665		0.842	1.407			
		34.46	////		74.12 / 74.12	////			135.00			0.665		0.842	1.407			
BC4	Lower-Chord	41.00	////		84.15 / 84.15	////	1.20		157.50			0.665		0.711	1.188			
		41.00	////		84.15 / 84.15	////	1.20		157.50			0.665		0.711	1.188			
		41.00	////		84.15 / 84.15	////			157.50			0.665		0.857	1.432			
		41.00	////		84.15 / 84.15	////			157.50			0.665		0.857	1.432			
BC5	Lower-Chord	34.46	////		74.12 / 74.12	////	1.20		135.00			0.665		0.699	1.168			
		34.46	////		74.12 / 74.12	////	1.20		135.00			0.665		0.699	1.168			
		34.46	////		74.12 / 74.12	////			135.00			0.665		0.842	1.407			
		34.46	////		74.12 / 74.12	////			135.00			0.665		0.842	1.407			
BC6	Lower-Chord	20.61	////		45.39 / 45.39	////	1.20		93.75			0.665		0.848	1.415			
		20.61	////		45.39 / 45.39	////	1.20		93.75			0.665		0.848	1.415			
		20.61	////		45.39 / 45.39	////			93.75			0.665		1.021	1.705			
		20.61	////		45.39 / 45.39	////			93.75			0.665		1.021	1.705			
BC7	Lower-Chord	20.61	////		45.39 / 45.39	////	1.20		88.02			0.665		0.775	1.294			
		20.61	////		45.39 / 45.39	////	1.20		88.02			0.665		0.775	1.294			
		20.61	////		45.39 / 45.39	////			88.02			0.665		0.934	1.559			
		20.61	////		45.39 / 45.39	////			88.02			0.665		0.934	1.559			
TC2	Upper-Chord	-34.46	-74.12 / -74.12	1.20	////	////		-232.63				0.665		1.456	2.431			
		-34.46	-74.12 / -74.12	1.20	////	////		-232.63				0.665		1.456	2.431			
		-34.46	-74.12 / -74.12	1.20	////	////		-232.63				0.665		1.754	2.929			
		-34.46	-74.12 / -74.12	1.20	////	////		-232.63				0.665		1.754	2.929			
TC3	Upper-Chord	-41.45	-87.73 / -87.73	1.20	////	////		-232.63				0.665		1.171	1.955			
		-41.45	-87.73 / -87.73	1.20	////	////		-232.63				0.665		1.171	1.955			
		-41.45	-87.73 / -87.73	1.20	////	////		-232.63				0.665		1.410	2.355			
		-41.45	-87.73 / -87.73	1.20	////	////		-232.63				0.665		1.410	2.355			
TC4	Upper-Chord	-41.91	-87.41 / -87.20	1.20	////	////		-232.63				0.665		1.171	1.960			
		-41.91	-87.41 / -87.20	1.20	////	////		-232.63				0.665		1.171	1.960			
		-41.91	-87.41 / -87.20	1.20	////	////		-232.63				0.665		1.411	2.362			
		-41.91	-87.41 / -87.20	1.20	////	////		-232.63				0.665		1.411	2.362			
TC5	Upper-Chord	-41.45	-87.73 / -87.73	1.20	////	////		-232.63				0.665		1.171	1.955			
		-41.45	-87.73 / -87.73	1.20	////	////		-232.63				0.665		1.171	1.955			
		-41.45	-87.73 / -87.73	1.20	////	////		-232.63				0.665		1.410	2.355			
		-41.45	-87.73 / -87.73	1.20	////	////		-232.63				0.665		1.410	2.355			
TC6	Upper-Chord	-34.46	-74.12 / -74.12	1.20	////	////		-232.63				0.665		1.456	2.431			
		-34.46	-74.12 / -74.12	1.20	////	////		-232.63				0.665		1.456	2.431			
		-34.46	-74.12 / -74.12	1.20	////	////		-232.63				0.665		1.754	2.929			
		-34.46	-74.12 / -74.12	1.20	////	////		-232.63				0.665		1.754	2.929			
VW1	Vertical	7.18	////		39.06 / 39.06	////	1.30		41.34			0.665		0.436	0.729			

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.18		////		39.06 / 39.06 ///	1.30		41.34			0.665	0.436	0.729			
		7.18		////		39.06 / 39.06 ///			41.34			0.665	0.567	0.947			
		7.18		////		39.06 / 39.06 ///			41.34			0.665	0.567	0.947			
VW2	Vertical	-9.67		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.877	1.465			
		-9.67		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.877	1.465			
		-9.67		-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.092	1.824			
		-9.67		-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.092	1.824			
VW3L	Vertical	-1.58		-24.82 / -24.56 ///	1.27	0.39 / 0.65 ///	1.27	-41.15	86.61			0.665	0.859	1.449			
		-1.58		-24.82 / -24.56 ///	1.27	0.39 / 0.65 ///	1.27	-41.15	86.61			0.665	0.859	1.449			
		-1.58		-24.82 / -24.56 ///		0.39 / 0.65 ///		-41.15	86.61			0.665	1.090	1.840			
		-1.58		-24.82 / -24.56 ///		0.39 / 0.65 ///		-41.15	86.61			0.665	1.090	1.840			
VW4	Vertical	-1.58		-24.82 / -24.56 ///	1.27	0.39 / 0.65 ///	1.27	-68.58	86.61			0.665	1.461	2.466			
		-1.58		-24.82 / -24.56 ///	1.27	0.39 / 0.65 ///	1.27	-68.58	86.61			0.665	1.461	2.466			
		-1.58		-24.82 / -24.56 ///		0.39 / 0.65 ///		-68.58	86.61			0.665	1.855	3.132			
		-1.58		-24.82 / -24.56 ///		0.39 / 0.65 ///		-68.58	86.61			0.665	1.855	3.132			
VW5	Vertical	-9.67		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.877	1.465			
		-9.67		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.877	1.465			
		-9.67		-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.092	1.824			
		-9.67		-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.092	1.824			
VW6	Vertical	7.18		////		39.06 / 39.06 ///	1.30		41.34			0.665	0.436	0.729			
		7.18		////		39.06 / 39.06 ///	1.30		41.34			0.665	0.436	0.729			
		7.18		////		39.06 / 39.06 ///			41.34			0.665	0.567	0.947			
		7.18		////		39.06 / 39.06 ///			41.34			0.665	0.567	0.947			
TC1	Diagonal	-32.76		-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.266	2.114			
		-32.76		-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.266	2.114			
		-32.76		-72.14 / -72.14 ///		////	////	-201.57				0.665	1.526	2.548			
		-32.76		-72.14 / -72.14 ///		////	////	-201.57				0.665	1.526	2.548			
TC7	Diagonal	-32.76		-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.266	2.114			
		-32.76		-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.266	2.114			
		-32.76		-72.14 / -72.14 ///		////	////	-201.57				0.665	1.526	2.548			
		-32.76		-72.14 / -72.14 ///		////	////	-201.57				0.665	1.526	2.548			
DW1	Diagonal	22.01		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		84.38			0.665	0.536	0.895			
		22.01		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		84.38			0.665	0.536	0.895			
		22.01		-6.92 / -6.92 ///		58.90 / 58.90 ///			84.38			0.665	0.655	1.094			
		22.01		-6.92 / -6.92 ///		58.90 / 58.90 ///			84.38			0.665	0.655	1.094			
DW2	Diagonal	11.11		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		54.00			0.665	0.482	0.804			
		11.11		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		54.00			0.665	0.482	0.804			
		11.11		-19.20 / -19.20 ///		45.67 / 45.67 ///			54.00			0.665	0.600	1.001			
		11.11		-19.20 / -19.20 ///		45.67 / 45.67 ///			54.00			0.665	0.600	1.001			
DW3	Diagonal	0.72		-0.49 / -0.81 ///	1.27	31.93 / 31.60 ///	1.27		41.34			0.665	0.690	1.164			
		0.72		-0.49 / -0.81 ///	1.27	31.93 / 31.60 ///	1.27		41.34			0.665	0.690	1.164			
		0.72		-0.49 / -0.81 ///		31.93 / 31.60 ///			41.34			0.665	0.876	1.478			
		0.72		-0.49 / -0.81 ///		31.93 / 31.60 ///			41.34			0.665	0.876	1.478			
DW4	Diagonal	0.72		-0.49 / -0.81 ///	1.27	31.93 / 31.60 ///	1.27		41.34			0.665	0.690	1.164			
		0.72		-0.49 / -0.81 ///	1.27	31.93 / 31.60 ///	1.27		41.34			0.665	0.690	1.164			
		0.72		-0.49 / -0.81 ///		31.93 / 31.60 ///			41.34			0.665	0.876	1.478			
		0.72		-0.49 / -0.81 ///		31.93 / 31.60 ///			41.34			0.665	0.876	1.478			
DW5	Diagonal	11.11		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		54.00			0.665	0.482	0.804			
		11.11		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		54.00			0.665	0.482	0.804			
		11.11		-19.20 / -19.20 ///		45.67 / 45.67 ///			54.00			0.665	0.600	1.001			
		11.11		-19.20 / -19.20 ///		45.67 / 45.67 ///			54.00			0.665	0.600	1.001			
DW6	Diagonal	22.01		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		84.38			0.665	0.536	0.895			
		22.01		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		84.38			0.665	0.536	0.895			
		22.01		-6.92 / -6.92 ///		58.90 / 58.90 ///			84.38			0.665	0.655	1.094			
		22.01		-6.92 / -6.92 ///		58.90 / 58.90 ///			84.38			0.665	0.655	1.094			

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: HS 20-44 (Design Lane)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand								
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF		
			Permit	Opr	Permit	Opr	Permit	Opr	Permit	Opr									
BC1	Lower-Chord	20.61		////		38.91 / 38.91	////	1.20		88.02			0.665		0.904	1.510			
		20.61		////		38.91 / 38.91	////	1.20		88.02			0.665		0.904	1.510			
		20.61		////		38.91 / 38.91	////			88.02			0.665		1.089	1.819			
		20.61		////		38.91 / 38.91	////			88.02			0.665		1.089	1.819			
BC2	Lower-Chord	20.61		////		38.91 / 38.91	////	1.20		93.75			0.665		0.989	1.651			
		20.61		////		38.91 / 38.91	////	1.20		93.75			0.665		0.989	1.651			
		20.61		////		38.91 / 38.91	////			93.75			0.665		1.191	1.989			
		20.61		////		38.91 / 38.91	////			93.75			0.665		1.191	1.989			
BC3	Lower-Chord	34.46		////		64.85 / 64.85	////	1.20		135.00			0.665		0.799	1.334			
		34.46		////		64.85 / 64.85	////	1.20		135.00			0.665		0.799	1.334			
		34.46		////		64.85 / 64.85	////			135.00			0.665		0.963	1.608			
		34.46		////		64.85 / 64.85	////			135.00			0.665		0.963	1.608			
BC4	Lower-Chord	41.00		////		71.90 / 72.11	////	1.20		157.50			0.665		0.833	1.386			
		41.00		////		71.90 / 72.11	////	1.20		157.50			0.665		0.833	1.386			
		41.00		////		71.90 / 72.11	////			157.50			0.665		1.003	1.671			
		41.00		////		71.90 / 72.11	////			157.50			0.665		1.003	1.671			
BC5	Lower-Chord	34.46		////		64.85 / 64.85	////	1.20		135.00			0.665		0.799	1.334			
		34.46		////		64.85 / 64.85	////	1.20		135.00			0.665		0.799	1.334			
		34.46		////		64.85 / 64.85	////			135.00			0.665		0.963	1.608			
		34.46		////		64.85 / 64.85	////			135.00			0.665		0.963	1.608			
BC6	Lower-Chord	20.61		////		38.91 / 38.91	////	1.20		93.75			0.665		0.989	1.651			
		20.61		////		38.91 / 38.91	////	1.20		93.75			0.665		0.989	1.651			
		20.61		////		38.91 / 38.91	////			93.75			0.665		1.191	1.989			
		20.61		////		38.91 / 38.91	////			93.75			0.665		1.191	1.989			
BC7	Lower-Chord	20.61		////		38.91 / 38.91	////	1.20		88.02			0.665		0.904	1.510			
		20.61		////		38.91 / 38.91	////	1.20		88.02			0.665		0.904	1.510			
		20.61		////		38.91 / 38.91	////			88.02			0.665		1.089	1.819			
		20.61		////		38.91 / 38.91	////			88.02			0.665		1.089	1.819			
TC2	Upper-Chord	-34.46		-64.85 / -64.85	////	1.20		////		-232.63			0.665		1.664	2.779			
		-34.46		-64.85 / -64.85	////	1.20		////		-232.63			0.665		1.664	2.779			
		-34.46		-64.85 / -64.85	////			////		-232.63			0.665		2.005	3.348			
		-34.46		-64.85 / -64.85	////			////		-232.63			0.665		2.005	3.348			
TC3	Upper-Chord	-41.45		-77.83 / -77.83	////	1.20		////		-232.63			0.665		1.319	2.204			
		-41.45		-77.83 / -77.83	////	1.20		////		-232.63			0.665		1.319	2.204			
		-41.45		-77.83 / -77.83	////			////		-232.63			0.665		1.590	2.655			
		-41.45		-77.83 / -77.83	////			////		-232.63			0.665		1.590	2.655			
TC4	Upper-Chord	-41.91		-77.51 / -77.30	////	1.20		////		-232.63			0.665		1.320	2.211			
		-41.91		-77.51 / -77.30	////	1.20		////		-232.63			0.665		1.320	2.211			
		-41.91		-77.51 / -77.30	////			////		-232.63			0.665		1.591	2.664			
		-41.91		-77.51 / -77.30	////			////		-232.63			0.665		1.591	2.664			
TC5	Upper-Chord	-41.45		-77.83 / -77.83	////	1.20		////		-232.63			0.665		1.319	2.204			
		-41.45		-77.83 / -77.83	////	1.20		////		-232.63			0.665		1.319	2.204			
		-41.45		-77.83 / -77.83	////			////		-232.63			0.665		1.590	2.655			
		-41.45		-77.83 / -77.83	////			////		-232.63			0.665		1.590	2.655			
TC6	Upper-Chord	-34.46		-64.85 / -64.85	////	1.20		////		-232.63			0.665		1.664	2.779			
		-34.46		-64.85 / -64.85	////	1.20		////		-232.63			0.665		1.664	2.779			
		-34.46		-64.85 / -64.85	////			////		-232.63			0.665		2.005	3.348			
		-34.46		-64.85 / -64.85	////			////		-232.63			0.665		2.005	3.348			
VW1	Vertical	7.18		////				36.88 / 36.88	////	1.30		41.34			0.665		0.462	0.772	

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.18		////		36.88 / 36.88	1.30		41.34			0.665	0.462	0.772			
		7.18		////		36.88 / 36.88	1.30		41.34			0.665	0.601	1.003			
		7.18		////		36.88 / 36.88	1.30		41.34			0.665	0.601	1.003			
VW2	Vertical	-9.67		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.061	1.771			
		-9.67		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.061	1.771			
		-9.67		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.320	2.205			
		-9.67		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.320	2.205			
VW3L	Vertical	-1.58		-18.90 / -18.64	1.27	0.39 / 0.65	1.27	-41.15	86.61			0.665	1.128	1.910			
		-1.58		-18.90 / -18.64	1.27	0.39 / 0.65	1.27	-41.15	86.61			0.665	1.128	1.910			
		-1.58		-18.90 / -18.64	1.27	0.39 / 0.65	1.27	-41.15	86.61			0.665	1.432	2.425			
		-1.58		-18.90 / -18.64	1.27	0.39 / 0.65	1.27	-41.15	86.61			0.665	1.432	2.425			
VW4	Vertical	-1.58		-18.90 / -18.64	1.27	0.39 / 0.65	1.27	-68.58	86.61			0.665	1.919	3.249			
		-1.58		-18.90 / -18.64	1.27	0.39 / 0.65	1.27	-68.58	86.61			0.665	1.919	3.249			
		-1.58		-18.90 / -18.64	1.27	0.39 / 0.65	1.27	-68.58	86.61			0.665	2.436	4.126			
		-1.58		-18.90 / -18.64	1.27	0.39 / 0.65	1.27	-68.58	86.61			0.665	2.436	4.126			
VW5	Vertical	-9.67		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.061	1.771			
		-9.67		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.061	1.771			
		-9.67		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.320	2.205			
		-9.67		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.320	2.205			
VW6	Vertical	7.18		////		36.88 / 36.88	1.30		41.34			0.665	0.462	0.772			
		7.18		////		36.88 / 36.88	1.30		41.34			0.665	0.462	0.772			
		7.18		////		36.88 / 36.88	1.30		41.34			0.665	0.601	1.003			
		7.18		////		36.88 / 36.88	1.30		41.34			0.665	0.601	1.003			
TC1	Diagonal	-32.76		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.292	2.158			
		-32.76		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.292	2.158			
		-32.76		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.557	2.601			
		-32.76		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.557	2.601			
TC7	Diagonal	-32.76		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.292	2.158			
		-32.76		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.292	2.158			
		-32.76		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.557	2.601			
		-32.76		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.557	2.601			
DW1	Diagonal	22.01		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.595	0.993			
		22.01		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.595	0.993			
		22.01		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.728	1.215			
		22.01		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.728	1.215			
DW2	Diagonal	11.11		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.582	0.972			
		11.11		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.582	0.972			
		11.11		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.725	1.210			
		11.11		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.725	1.210			
DW3	Diagonal	0.72		////		24.32 / 23.98	1.27		41.34			0.665	0.906	1.534			
		0.72		////		24.32 / 23.98	1.27		41.34			0.665	0.906	1.534			
		0.72		////		24.32 / 23.98	1.27		41.34			0.665	1.150	1.947			
		0.72		////		24.32 / 23.98	1.27		41.34			0.665	1.150	1.947			
DW4	Diagonal	0.72		////		24.32 / 23.98	1.27		41.34			0.665	0.906	1.534			
		0.72		////		24.32 / 23.98	1.27		41.34			0.665	0.906	1.534			
		0.72		////		24.32 / 23.98	1.27		41.34			0.665	1.150	1.947			
		0.72		////		24.32 / 23.98	1.27		41.34			0.665	1.150	1.947			
DW5	Diagonal	11.11		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.582	0.972			
		11.11		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.582	0.972			
		11.11		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.725	1.210			
		11.11		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.725	1.210			
DW6	Diagonal	22.01		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.595	0.993			
		22.01		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.595	0.993			
		22.01		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.728	1.215			
		22.01		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.728	1.215			

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: Type 3 (Design Truck)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand							
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		IF	Comp. (kip)	Tens. (kip)	Comp. (kip)	Tens. (kip)	One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF
			Permit	Opr	Permit	Opr												
BC1	Lower-Chord	20.61	////		32.16 / 32.16	1.20		88.02			0.665		1.094	1.826				
		20.61	////		32.16 / 32.16	1.20		88.02			0.665		1.094	1.826				
		20.61	////		32.16 / 32.16			88.02			0.665		1.318	2.201				
		20.61	////		32.16 / 32.16			88.02			0.665		1.318	2.201				
BC2	Lower-Chord	20.61	////		32.16 / 32.16	1.20		93.75			0.665		1.196	1.997				
		20.61	////		32.16 / 32.16	1.20		93.75			0.665		1.196	1.997				
		20.61	////		32.16 / 32.16			93.75			0.665		1.441	2.407				
		20.61	////		32.16 / 32.16			93.75			0.665		1.441	2.407				
BC3	Lower-Chord	34.46	////		52.76 / 52.76	1.20		135.00			0.665		0.982	1.640				
		34.46	////		52.76 / 52.76	1.20		135.00			0.665		0.982	1.640				
		34.46	////		52.76 / 52.76			135.00			0.665		1.183	1.976				
		34.46	////		52.76 / 52.76			135.00			0.665		1.183	1.976				
BC4	Lower-Chord	41.00	////		58.42 / 58.64	1.20		157.50			0.665		1.025	1.705				
		41.00	////		58.42 / 58.64	1.20		157.50			0.665		1.025	1.705				
		41.00	////		58.42 / 58.64			157.50			0.665		1.235	2.054				
		41.00	////		58.42 / 58.64			157.50			0.665		1.235	2.054				
BC5	Lower-Chord	34.46	////		52.76 / 52.76	1.20		135.00			0.665		0.982	1.640				
		34.46	////		52.76 / 52.76	1.20		135.00			0.665		0.982	1.640				
		34.46	////		52.76 / 52.76			135.00			0.665		1.183	1.976				
		34.46	////		52.76 / 52.76			135.00			0.665		1.183	1.976				
BC6	Lower-Chord	20.61	////		32.16 / 32.16	1.20		93.75			0.665		1.196	1.997				
		20.61	////		32.16 / 32.16	1.20		93.75			0.665		1.196	1.997				
		20.61	////		32.16 / 32.16			93.75			0.665		1.441	2.407				
		20.61	////		32.16 / 32.16			93.75			0.665		1.441	2.407				
BC7	Lower-Chord	20.61	////		32.16 / 32.16	1.20		88.02			0.665		1.094	1.826				
		20.61	////		32.16 / 32.16	1.20		88.02			0.665		1.094	1.826				
		20.61	////		32.16 / 32.16			88.02			0.665		1.318	2.201				
		20.61	////		32.16 / 32.16			88.02			0.665		1.318	2.201				
TC2	Upper-Chord	-34.46	-52.76 / -52.76	1.20	////			-232.63			0.665		2.045	3.416				
		-34.46	-52.76 / -52.76	1.20	////			-232.63			0.665		2.045	3.416				
		-34.46	-52.76 / -52.76		////			-232.63			0.665		2.464	4.115				
		-34.46	-52.76 / -52.76		////			-232.63			0.665		2.464	4.115				
TC3	Upper-Chord	-41.45	-62.64 / -62.64	1.20	////			-232.63			0.665		1.639	2.738				
		-41.45	-62.64 / -62.64	1.20	////			-232.63			0.665		1.639	2.738				
		-41.45	-62.64 / -62.64		////			-232.63			0.665		1.975	3.299				
		-41.45	-62.64 / -62.64		////			-232.63			0.665		1.975	3.299				
TC4	Upper-Chord	-41.91	-62.32 / -62.11	1.20	////			-232.63			0.665		1.642	2.752				
		-41.91	-62.32 / -62.11	1.20	////			-232.63			0.665		1.642	2.752				
		-41.91	-62.32 / -62.11		////			-232.63			0.665		1.979	3.316				
		-41.91	-62.32 / -62.11		////			-232.63			0.665		1.979	3.316				
TC5	Upper-Chord	-41.45	-62.64 / -62.64	1.20	////			-232.63			0.665		1.639	2.738				
		-41.45	-62.64 / -62.64	1.20	////			-232.63			0.665		1.639	2.738				
		-41.45	-62.64 / -62.64		////			-232.63			0.665		1.975	3.299				
		-41.45	-62.64 / -62.64		////			-232.63			0.665		1.975	3.299				
TC6	Upper-Chord	-34.46	-52.76 / -52.76	1.20	////			-232.63			0.665		2.045	3.416				
		-34.46	-52.76 / -52.76	1.20	////			-232.63			0.665		2.045	3.416				
		-34.46	-52.76 / -52.76		////			-232.63			0.665		2.464	4.115				
		-34.46	-52.76 / -52.76		////			-232.63			0.665		2.464	4.115				
VW1	Vertical	7.18	////		31.88 / 31.88	1.30		41.34			0.665		0.535	0.893				

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.18		////		31.88 / 31.88	1.30		41.34			0.665	0.535	0.893			
		7.18		////		31.88 / 31.88	1.30		41.34			0.665	0.695	1.160			
		7.18		////		31.88 / 31.88	1.30		41.34			0.665	0.695	1.160			
VW2	Vertical	-9.67		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665		1.224	2.044		
		-9.67		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665		1.224	2.044		
		-9.67		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665		1.524	2.544		
		-9.67		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665		1.524	2.544		
VW3L	Vertical	-1.58		-17.91 / -17.65	1.27	0.39 / 0.65	1.27	-41.15	86.61			0.665		1.190	2.017		
		-1.58		-17.91 / -17.65	1.27	0.39 / 0.65	1.27	-41.15	86.61			0.665		1.190	2.017		
		-1.58		-17.91 / -17.65	1.27	0.39 / 0.65	1.27	-41.15	86.61			0.665		1.511	2.561		
		-1.58		-17.91 / -17.65	1.27	0.39 / 0.65	1.27	-41.15	86.61			0.665		1.511	2.561		
VW4	Vertical	-1.58		-17.91 / -17.65	1.27	0.39 / 0.65	1.27	-68.58	86.61			0.665		2.025	3.431		
		-1.58		-17.91 / -17.65	1.27	0.39 / 0.65	1.27	-68.58	86.61			0.665		2.025	3.431		
		-1.58		-17.91 / -17.65	1.27	0.39 / 0.65	1.27	-68.58	86.61			0.665		2.571	4.357		
		-1.58		-17.91 / -17.65	1.27	0.39 / 0.65	1.27	-68.58	86.61			0.665		2.571	4.357		
VW5	Vertical	-9.67		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665		1.224	2.044		
		-9.67		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665		1.224	2.044		
		-9.67		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665		1.524	2.544		
		-9.67		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665		1.524	2.544		
VW6	Vertical	7.18		////		31.88 / 31.88	1.30		41.34			0.665	0.535	0.893			
		7.18		////		31.88 / 31.88	1.30		41.34			0.665	0.535	0.893			
		7.18		////		31.88 / 31.88	1.30		41.34			0.665	0.695	1.160			
		7.18		////		31.88 / 31.88	1.30		41.34			0.665	0.695	1.160			
TC1	Diagonal	-32.76		-51.12 / -51.12	1.20	////	////	-201.57				0.665		1.787	2.984		
		-32.76		-51.12 / -51.12	1.20	////	////	-201.57				0.665		1.787	2.984		
		-32.76		-51.12 / -51.12	1.20	////	////	-201.57				0.665		2.153	3.595		
		-32.76		-51.12 / -51.12	1.20	////	////	-201.57				0.665		2.153	3.595		
TC7	Diagonal	-32.76		-51.12 / -51.12	1.20	////	////	-201.57				0.665		1.787	2.984		
		-32.76		-51.12 / -51.12	1.20	////	////	-201.57				0.665		1.787	2.984		
		-32.76		-51.12 / -51.12	1.20	////	////	-201.57				0.665		2.153	3.595		
		-32.76		-51.12 / -51.12	1.20	////	////	-201.57				0.665		2.153	3.595		
DW1	Diagonal	22.01		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665		0.753	1.257		
		22.01		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665		0.753	1.257		
		22.01		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665		0.921	1.538		
		22.01		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665		0.921	1.538		
DW2	Diagonal	11.11		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665		0.672	1.122		
		11.11		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665		0.672	1.122		
		11.11		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665		0.836	1.397		
		11.11		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665		0.836	1.397		
DW3	Diagonal	0.72		-0.33 / -0.33	1.27	23.05 / 22.71	1.27		41.34			0.665		0.956	1.619		
		0.72		-0.33 / -0.33	1.27	23.05 / 22.71	1.27		41.34			0.665		0.956	1.619		
		0.72		-0.33 / -0.33	1.27	23.05 / 22.71	1.27		41.34			0.665		1.213	2.056		
		0.72		-0.33 / -0.33	1.27	23.05 / 22.71	1.27		41.34			0.665		1.213	2.056		
DW4	Diagonal	0.72		-0.33 / -0.33	1.27	23.05 / 22.71	1.27		41.34			0.665		0.956	1.619		
		0.72		-0.33 / -0.33	1.27	23.05 / 22.71	1.27		41.34			0.665		0.956	1.619		
		0.72		-0.33 / -0.33	1.27	23.05 / 22.71	1.27		41.34			0.665		1.213	2.056		
		0.72		-0.33 / -0.33	1.27	23.05 / 22.71	1.27		41.34			0.665		1.213	2.056		
DW5	Diagonal	11.11		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665		0.672	1.122		
		11.11		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665		0.672	1.122		
		11.11		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665		0.836	1.397		
		11.11		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665		0.836	1.397		
DW6	Diagonal	22.01		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665		0.753	1.257		
		22.01		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665		0.753	1.257		
		22.01		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665		0.921	1.538		
		22.01		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665		0.921	1.538		

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: Type 3-3 (Design Truck)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand							
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		IF	Comp. (kip)	Tens. (kip)	Comp. (kip)	Tens. (kip)	One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF
			Permit	Opr	Permit	Opr												
BC1	Lower-Chord	20.61	////		42.50 / 42.50	1.20		88.02			0.665		0.828	1.382				
		20.61	////		42.50 / 42.50	1.20		88.02			0.665		0.828	1.382				
		20.61	////		42.50 / 42.50			88.02			0.665		0.997	1.665				
		20.61	////		42.50 / 42.50			88.02			0.665		0.997	1.665				
BC2	Lower-Chord	20.61	////		42.50 / 42.50	1.20		93.75			0.665		0.905	1.511				
		20.61	////		42.50 / 42.50	1.20		93.75			0.665	0.665	0.905	1.511				
		20.61	////		42.50 / 42.50			93.75			0.665		1.090	1.821				
		20.61	////		42.50 / 42.50			93.75			0.665		1.090	1.821				
BC3	Lower-Chord	34.46	////		68.19 / 68.19	1.20		135.00			0.665		0.760	1.269				
		34.46	////		68.19 / 68.19	1.20		135.00			0.665		0.760	1.269				
		34.46	////		68.19 / 68.19			135.00			0.665		0.916	1.529				
		34.46	////		68.19 / 68.19			135.00			0.665		0.916	1.529				
BC4	Lower-Chord	41.00	////		78.80 / 78.80	1.20		157.50			0.665		0.760	1.269				
		41.00	////		78.80 / 78.80	1.20		157.50			0.665		0.760	1.269				
		41.00	////		78.80 / 78.80			157.50			0.665		0.915	1.529				
		41.00	////		78.80 / 78.80			157.50			0.665		0.915	1.529				
BC5	Lower-Chord	34.46	////		68.19 / 68.19	1.20		135.00			0.665		0.760	1.269				
		34.46	////		68.19 / 68.19	1.20		135.00			0.665		0.760	1.269				
		34.46	////		68.19 / 68.19			135.00			0.665		0.916	1.529				
		34.46	////		68.19 / 68.19			135.00			0.665		0.916	1.529				
BC6	Lower-Chord	20.61	////		42.50 / 42.50	1.20		93.75			0.665		0.905	1.511				
		20.61	////		42.50 / 42.50	1.20		93.75			0.665		0.905	1.511				
		20.61	////		42.50 / 42.50			93.75			0.665	0.665	1.090	1.821				
		20.61	////		42.50 / 42.50			93.75			0.665		1.090	1.821				
BC7	Lower-Chord	20.61	////		42.50 / 42.50	1.20		88.02			0.665		0.828	1.382				
		20.61	////		42.50 / 42.50	1.20		88.02			0.665		0.828	1.382				
		20.61	////		42.50 / 42.50			88.02			0.665		0.997	1.665				
		20.61	////		42.50 / 42.50			88.02			0.665		0.997	1.665				
TC2	Upper-Chord	-34.46	-68.19 / -68.19	1.20	////		-232.63				0.665		1.582	2.643				
		-34.46	-68.19 / -68.19	1.20	////		-232.63				0.665		1.582	2.643				
		-34.46	-68.19 / -68.19		////		-232.63				0.665		1.907	3.184				
		-34.46	-68.19 / -68.19		////		-232.63				0.665		1.907	3.184				
TC3	Upper-Chord	-41.45	-80.65 / -80.65	1.20	////		-232.63				0.665		1.273	2.126				
		-41.45	-80.65 / -80.65	1.20	////		-232.63				0.665		1.273	2.126				
		-41.45	-80.65 / -80.65		////		-232.63				0.665		1.534	2.562				
		-41.45	-80.65 / -80.65		////		-232.63				0.665		1.534	2.562				
TC4	Upper-Chord	-41.91	-81.01 / -81.01	1.20	////		-232.63				0.665		1.263	2.110				
		-41.91	-81.01 / -81.01	1.20	////		-232.63				0.665	0.665	1.263	2.110				
		-41.91	-81.01 / -81.01		////		-232.63				0.665		1.522	2.542				
		-41.91	-81.01 / -81.01		////		-232.63				0.665		1.522	2.542				
TC5	Upper-Chord	-41.45	-80.65 / -80.65	1.20	////		-232.63				0.665		1.273	2.126				
		-41.45	-80.65 / -80.65	1.20	////		-232.63				0.665		1.273	2.126				
		-41.45	-80.65 / -80.65		////		-232.63				0.665		1.534	2.562				
		-41.45	-80.65 / -80.65		////		-232.63				0.665		1.534	2.562				
TC6	Upper-Chord	-34.46	-68.19 / -68.19	1.20	////		-232.63				0.665		1.582	2.643				
		-34.46	-68.19 / -68.19	1.20	////		-232.63				0.665		1.582	2.643				
		-34.46	-68.19 / -68.19		////		-232.63				0.665		1.907	3.184				
		-34.46	-68.19 / -68.19		////		-232.63				0.665		1.907	3.184				
VW1	Vertical	7.18	////		25.65 / 25.65	1.30		41.34			0.665		0.664	1.110				

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.18	////		25.65 / 25.65 ///	1.30		41.34			0.665	0.664	1.110		
		7.18	////		25.65 / 25.65 ///			41.34			0.665	0.864	1.443		
		7.18	////		25.65 / 25.65 ///			41.34			0.665	0.864	1.443		
VW2	Vertical	-9.67	-29.65 / -29.65 ///	1.24	9.41 / 9.41 ///	1.30	-68.58	86.61			0.665		1.051	1.754	
		-9.67	-29.65 / -29.65 ///	1.24	9.41 / 9.41 ///	1.30	-68.58	86.61			0.665		1.051	1.754	
		-9.67	-29.65 / -29.65 ///		9.41 / 9.41 ///		-68.58	86.61			0.665		1.308	2.184	
		-9.67	-29.65 / -29.65 ///		9.41 / 9.41 ///		-68.58	86.61			0.665		1.308	2.184	
VW3L	Vertical	-1.58	-18.13 / -17.87 ///	1.27	0.39 / 0.65 ///	1.27	-41.15	86.61			0.665		1.176	1.992	
		-1.58	-18.13 / -17.87 ///	1.27	0.39 / 0.65 ///	1.27	-41.15	86.61			0.665		1.176	1.992	
		-1.58	-18.13 / -17.87 ///		0.39 / 0.65 ///		-41.15	86.61			0.665		1.493	2.529	
		-1.58	-18.13 / -17.87 ///		0.39 / 0.65 ///		-41.15	86.61			0.665		1.493	2.529	
VW4	Vertical	-1.58	-18.13 / -17.87 ///	1.27	0.39 / 0.65 ///	1.27	-68.58	86.61			0.665		2.000	3.389	
		-1.58	-18.13 / -17.87 ///	1.27	0.39 / 0.65 ///	1.27	-68.58	86.61			0.665		2.000	3.389	
		-1.58	-18.13 / -17.87 ///		0.39 / 0.65 ///		-68.58	86.61			0.665		2.540	4.304	
		-1.58	-18.13 / -17.87 ///		0.39 / 0.65 ///		-68.58	86.61			0.665		2.540	4.304	
VW5	Vertical	-9.67	-29.65 / -29.65 ///	1.24	9.41 / 9.41 ///	1.30	-68.58	86.61			0.665		1.051	1.754	
		-9.67	-29.65 / -29.65 ///	1.24	9.41 / 9.41 ///	1.30	-68.58	86.61			0.665		1.051	1.754	
		-9.67	-29.65 / -29.65 ///		9.41 / 9.41 ///		-68.58	86.61			0.665		1.308	2.184	
		-9.67	-29.65 / -29.65 ///		9.41 / 9.41 ///		-68.58	86.61			0.665		1.308	2.184	
VW6	Vertical	7.18	////		25.65 / 25.65 ///	1.30		41.34			0.665		0.664	1.110	
		7.18	////		25.65 / 25.65 ///	1.30		41.34			0.665		0.664	1.110	
		7.18	////		25.65 / 25.65 ///			41.34			0.665		0.864	1.443	
		7.18	////		25.65 / 25.65 ///			41.34			0.665		0.864	1.443	
TC1	Diagonal	-32.76	-67.55 / -67.55 ///	1.20	////	////	-201.57				0.665		1.352	2.258	
		-32.76	-67.55 / -67.55 ///	1.20	////	////	-201.57				0.665		1.352	2.258	
		-32.76	-67.55 / -67.55 ///		////	////	-201.57				0.665		1.629	2.721	
		-32.76	-67.55 / -67.55 ///		////	////	-201.57				0.665		1.629	2.721	
TC7	Diagonal	-32.76	-67.55 / -67.55 ///	1.20	////	////	-201.57				0.665		1.352	2.258	
		-32.76	-67.55 / -67.55 ///	1.20	////	////	-201.57				0.665		1.352	2.258	
		-32.76	-67.55 / -67.55 ///		////	////	-201.57				0.665		1.629	2.721	
		-32.76	-67.55 / -67.55 ///		////	////	-201.57				0.665		1.629	2.721	
DW1	Diagonal	22.01	-4.54 / -4.54 ///	1.30	52.85 / 52.85 ///	1.22		84.38			0.665		0.597	0.997	
		22.01	-4.54 / -4.54 ///	1.30	52.85 / 52.85 ///	1.22		84.38			0.665		0.597	0.997	
		22.01	-4.54 / -4.54 ///		52.85 / 52.85 ///			84.38			0.665		0.730	1.220	
		22.01	-4.54 / -4.54 ///		52.85 / 52.85 ///			84.38			0.665		0.730	1.220	
DW2	Diagonal	11.11	-12.11 / -12.11 ///	1.30	38.14 / 38.14 ///	1.24		54.00			0.665		0.577	0.963	
		11.11	-12.11 / -12.11 ///	1.30	38.14 / 38.14 ///	1.24		54.00			0.665		0.577	0.963	
		11.11	-12.11 / -12.11 ///		38.14 / 38.14 ///			54.00			0.665		0.718	1.199	
		11.11	-12.11 / -12.11 ///		38.14 / 38.14 ///			54.00			0.665		0.718	1.199	
DW3	Diagonal	0.72	-0.48 / -0.74 ///	1.27	23.33 / 22.99 ///	1.27		41.34			0.665		0.944	1.600	
		0.72	-0.48 / -0.74 ///	1.27	23.33 / 22.99 ///	1.27		41.34			0.665		0.944	1.600	
		0.72	-0.48 / -0.74 ///		23.33 / 22.99 ///			41.34			0.665		1.199	2.031	
		0.72	-0.48 / -0.74 ///		23.33 / 22.99 ///			41.34			0.665		1.199	2.031	
DW4	Diagonal	0.72	-0.48 / -0.74 ///	1.27	23.33 / 22.99 ///	1.27		41.34			0.665		0.944	1.600	
		0.72	-0.48 / -0.74 ///	1.27	23.33 / 22.99 ///	1.27		41.34			0.665		0.944	1.600	
		0.72	-0.48 / -0.74 ///		23.33 / 22.99 ///			41.34			0.665		1.199	2.031	
		0.72	-0.48 / -0.74 ///		23.33 / 22.99 ///			41.34			0.665		1.199	2.031	
DW5	Diagonal	11.11	-12.11 / -12.11 ///	1.30	38.14 / 38.14 ///	1.24		54.00			0.665		0.577	0.963	
		11.11	-12.11 / -12.11 ///	1.30	38.14 / 38.14 ///	1.24		54.00			0.665		0.577	0.963	
		11.11	-12.11 / -12.11 ///		38.14 / 38.14 ///			54.00			0.665		0.718	1.199	
		11.11	-12.11 / -12.11 ///		38.14 / 38.14 ///			54.00			0.665		0.718	1.199	
DW6	Diagonal	22.01	-4.54 / -4.54 ///	1.30	52.85 / 52.85 ///	1.22		84.38			0.665		0.597	0.997	
		22.01	-4.54 / -4.54 ///	1.30	52.85 / 52.85 ///	1.22		84.38			0.665		0.597	0.997	
		22.01	-4.54 / -4.54 ///		52.85 / 52.85 ///			84.38			0.665		0.730	1.220	
		22.01	-4.54 / -4.54 ///		52.85 / 52.85 ///			84.38			0.665		0.730	1.220	

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: Type 3S2 (Design Truck)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00

Inventory:

A1 = 1.30 , A2 = 2.17

Operating:

A1 = 1.30 , A2 = 1.30

Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand							
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF	
			Permit	Opr	Permit	Opr	Permit	Opr	Permit	Opr								
BC1	Lower-Chord	20.61		////		40.84 / 40.84	1.20		88.02			0.665		0.861	1.438			
		20.61		////		40.84 / 40.84	1.20		88.02			0.665		0.861	1.438			
		20.61		////		40.84 / 40.84			88.02			0.665		1.038	1.733			
		20.61		////		40.84 / 40.84			88.02			0.665		1.038	1.733			
BC2	Lower-Chord	20.61		////		40.84 / 40.84	1.20		93.75			0.665		0.942	1.573			
		20.61		////		40.84 / 40.84	1.20		93.75			0.665		0.942	1.573			
		20.61		////		40.84 / 40.84			93.75			0.665		1.135	1.895			
		20.61		////		40.84 / 40.84			93.75			0.665		1.135	1.895			
BC3	Lower-Chord	34.46		////		66.87 / 66.87	1.20		135.00			0.665		0.775	1.294			
		34.46		////		66.87 / 66.87	1.20		135.00			0.665		0.775	1.294			
		34.46		////		66.87 / 66.87			135.00			0.665		0.934	1.559			
		34.46		////		66.87 / 66.87			135.00			0.665		0.934	1.559			
BC4	Lower-Chord	41.00		////		75.78 / 75.99	1.20		157.50			0.665		0.790	1.316			
		41.00		////		75.78 / 75.99	1.20		157.50			0.665		0.790	1.316			
		41.00		////		75.78 / 75.99			157.50			0.665		0.952	1.585			
		41.00		////		75.78 / 75.99			157.50			0.665		0.952	1.585			
BC5	Lower-Chord	34.46		////		66.87 / 66.87	1.20		135.00			0.665		0.775	1.294			
		34.46		////		66.87 / 66.87	1.20		135.00			0.665		0.775	1.294			
		34.46		////		66.87 / 66.87			135.00			0.665		0.934	1.559			
		34.46		////		66.87 / 66.87			135.00			0.665		0.934	1.559			
BC6	Lower-Chord	20.61		////		40.84 / 40.84	1.20		93.75			0.665		0.942	1.573			
		20.61		////		40.84 / 40.84	1.20		93.75			0.665		0.942	1.573			
		20.61		////		40.84 / 40.84			93.75			0.665		1.135	1.895			
		20.61		////		40.84 / 40.84			93.75			0.665		1.135	1.895			
BC7	Lower-Chord	20.61		////		40.84 / 40.84	1.20		88.02			0.665		0.861	1.438			
		20.61		////		40.84 / 40.84	1.20		88.02			0.665		0.861	1.438			
		20.61		////		40.84 / 40.84			88.02			0.665		1.038	1.733			
		20.61		////		40.84 / 40.84			88.02			0.665		1.038	1.733			
TC2	Upper-Chord	-34.46		-66.87 / -66.87	1.20	////			-232.63			0.665		1.614	2.695			
		-34.46		-66.87 / -66.87	1.20	////			-232.63			0.665		1.614	2.695			
		-34.46		-66.87 / -66.87		////			-232.63			0.665		1.944	3.247			
		-34.46		-66.87 / -66.87		////			-232.63			0.665		1.944	3.247			
TC3	Upper-Chord	-41.45		-78.97 / -78.97	1.20	////			-232.63			0.665		1.300	2.172			
		-41.45		-78.97 / -78.97	1.20	////			-232.63			0.665		1.300	2.172			
		-41.45		-78.97 / -78.97		////			-232.63			0.665		1.567	2.617			
		-41.45		-78.97 / -78.97		////			-232.63			0.665		1.567	2.617			
TC4	Upper-Chord	-41.91		-78.65 / -78.44	1.20	////			-232.63			0.665		1.301	2.179			
		-41.91		-78.65 / -78.44	1.20	////			-232.63			0.665		1.301	2.179			
		-41.91		-78.65 / -78.44		////			-232.63			0.665		1.568	2.626			
		-41.91		-78.65 / -78.44		////			-232.63			0.665		1.568	2.626			
TC5	Upper-Chord	-41.45		-78.97 / -78.97	1.20	////			-232.63			0.665		1.300	2.172			
		-41.45		-78.97 / -78.97	1.20	////			-232.63			0.665		1.300	2.172			
		-41.45		-78.97 / -78.97		////			-232.63			0.665		1.567	2.617			
		-41.45		-78.97 / -78.97		////			-232.63			0.665		1.567	2.617			
TC6	Upper-Chord	-34.46		-66.87 / -66.87	1.20	////			-232.63			0.665		1.614	2.695			
		-34.46		-66.87 / -66.87	1.20	////			-232.63			0.665		1.614	2.695			
		-34.46		-66.87 / -66.87		////			-232.63			0.665		1.944	3.247			
		-34.46		-66.87 / -66.87		////			-232.63			0.665		1.944	3.247			
VW1	Vertical	7.18		////		30.88 / 30.88	1.30		41.34			0.665		0.552	0.922			

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.18	////		30.88 / 30.88 ///	1.30		41.34			0.665	0.552	0.922		
		7.18	////		30.88 / 30.88 ///			41.34			0.665	0.717	1.198		
		7.18	////		30.88 / 30.88 ///			41.34			0.665	0.717	1.198		
VW2	Vertical	-9.67	-29.88 / -29.88 ///	1.24	9.90 / 9.90 ///	1.30	-68.58	86.61			0.665	1.042	1.741		
		-9.67	-29.88 / -29.88 ///	1.24	9.90 / 9.90 ///	1.30	-68.58	86.61			0.665	1.042	1.741		
		-9.67	-29.88 / -29.88 ///		9.90 / 9.90 ///		-68.58	86.61			0.665	1.297	2.167		
		-9.67	-29.88 / -29.88 ///		9.90 / 9.90 ///		-68.58	86.61			0.665	1.297	2.167		
VW3L	Vertical	-1.58	-19.21 / -18.95 ///	1.27	0.39 / 0.65 ///	1.27	-41.15	86.61			0.665	1.110	1.879		
		-1.58	-19.21 / -18.95 ///	1.27	0.39 / 0.65 ///	1.27	-41.15	86.61			0.665	1.110	1.879		
		-1.58	-19.21 / -18.95 ///		0.39 / 0.65 ///		-41.15	86.61			0.665	1.409	2.386		
		-1.58	-19.21 / -18.95 ///		0.39 / 0.65 ///		-41.15	86.61			0.665	1.409	2.386		
VW4	Vertical	-1.58	-19.21 / -18.95 ///	1.27	0.39 / 0.65 ///	1.27	-68.58	86.61			0.665	1.888	3.197		
		-1.58	-19.21 / -18.95 ///	1.27	0.39 / 0.65 ///	1.27	-68.58	86.61			0.665	1.888	3.197		
		-1.58	-19.21 / -18.95 ///		0.39 / 0.65 ///		-68.58	86.61			0.665	2.398	4.059		
		-1.58	-19.21 / -18.95 ///		0.39 / 0.65 ///		-68.58	86.61			0.665	2.398	4.059		
VW5	Vertical	-9.67	-29.88 / -29.88 ///	1.24	9.90 / 9.90 ///	1.30	-68.58	86.61			0.665	1.042	1.741		
		-9.67	-29.88 / -29.88 ///	1.24	9.90 / 9.90 ///	1.30	-68.58	86.61			0.665	1.042	1.741		
		-9.67	-29.88 / -29.88 ///		9.90 / 9.90 ///		-68.58	86.61			0.665	1.297	2.167		
		-9.67	-29.88 / -29.88 ///		9.90 / 9.90 ///		-68.58	86.61			0.665	1.297	2.167		
VW6	Vertical	7.18	////		30.88 / 30.88 ///	1.30		41.34			0.665	0.552	0.922		
		7.18	////		30.88 / 30.88 ///	1.30		41.34			0.665	0.552	0.922		
		7.18	////		30.88 / 30.88 ///			41.34			0.665	0.717	1.198		
		7.18	////		30.88 / 30.88 ///			41.34			0.665	0.717	1.198		
TC1	Diagonal	-32.76	-64.91 / -64.91 ///	1.20	////	////	-201.57				0.665	1.407	2.350		
		-32.76	-64.91 / -64.91 ///	1.20	////	////	-201.57				0.665	1.407	2.350		
		-32.76	-64.91 / -64.91 ///		////	////	-201.57				0.665	1.695	2.831		
		-32.76	-64.91 / -64.91 ///		////	////	-201.57				0.665	1.695	2.831		
TC7	Diagonal	-32.76	-64.91 / -64.91 ///	1.20	////	////	-201.57				0.665	1.407	2.350		
		-32.76	-64.91 / -64.91 ///	1.20	////	////	-201.57				0.665	1.407	2.350		
		-32.76	-64.91 / -64.91 ///		////	////	-201.57				0.665	1.695	2.831		
		-32.76	-64.91 / -64.91 ///		////	////	-201.57				0.665	1.695	2.831		
DW1	Diagonal	22.01	-5.03 / -5.03 ///	1.30	51.68 / 51.68 ///	1.22		84.38			0.665	0.611	1.020		
		22.01	-5.03 / -5.03 ///	1.30	51.68 / 51.68 ///	1.22		84.38			0.665	0.611	1.020		
		22.01	-5.03 / -5.03 ///		51.68 / 51.68 ///			84.38			0.665	0.747	1.247		
		22.01	-5.03 / -5.03 ///		51.68 / 51.68 ///			84.38			0.665	0.747	1.247		
DW2	Diagonal	11.11	-12.74 / -12.74 ///	1.30	38.45 / 38.45 ///	1.24		54.00			0.665	0.572	0.956		
		11.11	-12.74 / -12.74 ///	1.30	38.45 / 38.45 ///	1.24		54.00			0.665	0.572	0.956		
		11.11	-12.74 / -12.74 ///		38.45 / 38.45 ///			54.00			0.665	0.712	1.189		
		11.11	-12.74 / -12.74 ///		38.45 / 38.45 ///			54.00			0.665	0.712	1.189		
DW3	Diagonal	0.72	-0.32 / -0.78 ///	1.27	24.71 / 24.38 ///	1.27		41.34			0.665	0.891	1.509		
		0.72	-0.32 / -0.78 ///	1.27	24.71 / 24.38 ///	1.27		41.34			0.665	0.891	1.509		
		0.72	-0.32 / -0.78 ///		24.71 / 24.38 ///			41.34			0.665	1.132	1.916		
		0.72	-0.32 / -0.78 ///		24.71 / 24.38 ///			41.34			0.665	1.132	1.916		
DW4	Diagonal	0.72	-0.32 / -0.78 ///	1.27	24.71 / 24.38 ///	1.27		41.34			0.665	0.891	1.509		
		0.72	-0.32 / -0.78 ///	1.27	24.71 / 24.38 ///	1.27		41.34			0.665	0.891	1.509		
		0.72	-0.32 / -0.78 ///		24.71 / 24.38 ///			41.34			0.665	1.132	1.916		
		0.72	-0.32 / -0.78 ///		24.71 / 24.38 ///			41.34			0.665	1.132	1.916		
DW5	Diagonal	11.11	-12.74 / -12.74 ///	1.30	38.45 / 38.45 ///	1.24		54.00			0.665	0.572	0.956		
		11.11	-12.74 / -12.74 ///	1.30	38.45 / 38.45 ///	1.24		54.00			0.665	0.572	0.956		
		11.11	-12.74 / -12.74 ///		38.45 / 38.45 ///			54.00			0.665	0.712	1.189		
		11.11	-12.74 / -12.74 ///		38.45 / 38.45 ///			54.00			0.665	0.712	1.189		
DW6	Diagonal	22.01	-5.03 / -5.03 ///	1.30	51.68 / 51.68 ///	1.22		84.38			0.665	0.611	1.020		
		22.01	-5.03 / -5.03 ///	1.30	51.68 / 51.68 ///	1.22		84.38			0.665	0.611	1.020		
		22.01	-5.03 / -5.03 ///		51.68 / 51.68 ///			84.38			0.665	0.747	1.247		
		22.01	-5.03 / -5.03 ///		51.68 / 51.68 ///			84.38			0.665	0.747	1.247		

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Bridge ID :L1567200002001
 Bridge : Swan River (Truss)
 StructDef : 119'SteelTruss(withReducedCapacityMembers)
 User : BrR
 Date : Thursday, December 04, 2014 09:06:45
 File : RatingResults.XML
 Analysis Preference Setting : None

NBI Structure ID :L15672000_02001
 Bridge Alt : As-built
 Member : SE Truss (Upstream)

Load Factor Rating Summary

Live Load	Live Load Type	Inv Element	Inv RF	Inv Capacity (Ton)	Opr Element	Opr RF	Opr Capacity (Ton)	Legal Opr Element	Legal Opr RF	Legal Opr Capacity (Ton)	Permit Inv Element	Permit Inv RF	Permit Inv Capacity (Ton)	Permit Opr Element	Permit Opr RF	Permit Opr Capacity (Ton)	Impact	Lane
HS 20-44	Design Truck	VW1	0.431	15.52	VW1	0.720	25.92										As Requested	As Requested
HS 20-44	Design Lane	VW1	0.457	16.44	VW1	0.763	27.45										As Requested	As Requested
HS 20-44	Design Truck	VW1	0.431	15.52	VW1	0.720	25.92										With Impact	Single Lane
HS 20-44	Design Lane	VW1	0.457	16.44	VW1	0.763	27.45										With Impact	Single Lane
HS 20-44	Design Truck	VW1	0.431	15.52	VW1	0.720	25.92										With Impact	Multi-Lane
HS 20-44	Design Lane	VW1	0.457	16.44	VW1	0.763	27.45										With Impact	Multi-Lane
HS 20-44	Design Truck	VW1	0.560	20.18	VW1	0.936	33.70										Without Impact	Single Lane
HS 20-44	Design Lane	VW1	0.594	21.37	VW1	0.991	35.69										Without Impact	Single Lane
HS 20-44	Design Truck	VW1	0.560	20.18	VW1	0.936	33.70										Without Impact	Multi-Lane
HS 20-44	Design Lane	VW1	0.594	21.37	VW1	0.991	35.69										Without Impact	Multi-Lane
Type 3	Design Truck	VW1	0.528	13.20	VW1	0.882	22.05										As Requested	As Requested
Type 3	Design Truck	VW1	0.528	13.20	VW1	0.882	22.05										With Impact	Single Lane
Type 3	Design Truck	VW1	0.528	13.20	VW1	0.882	22.05										With Impact	Multi-Lane
Type 3	Design Truck	VW1	0.687	17.17	VW1	1.147	28.67										Without Impact	Single Lane
Type 3	Design Truck	VW1	0.687	17.17	VW1	1.147	28.67										Without Impact	Multi-Lane
Type 3-3	Design Truck	DW2	0.569	22.77	DW2	0.951	38.03										As Requested	As Requested
Type 3-3	Design Truck	DW2	0.569	22.77	DW2	0.951	38.03										With Impact	Single Lane
Type 3-3	Design Truck	DW2	0.569	22.77	DW2	0.951	38.03										With Impact	Multi-Lane
Type 3-3	Design Truck	DW2	0.709	28.35	DW2	1.183	47.34										Without Impact	Single Lane
Type 3-3	Design Truck	DW2	0.709	28.35	DW2	1.183	47.34										Without Impact	Multi-Lane
Type 3S2	Design Truck	VW1	0.545	19.63	VW1	0.911	32.78										As Requested	As Requested
Type 3S2	Design Truck	VW1	0.545	19.63	VW1	0.911	32.78										With Impact	Single Lane
Type 3S2	Design Truck	VW1	0.545	19.63	VW1	0.911	32.78										With Impact	Multi-Lane
Type 3S2	Design Truck	DW2	0.703	25.31	DW2	1.174	42.27										Without Impact	Single Lane
Type 3S2	Design Truck	DW2	0.703	25.31	DW2	1.174	42.27										Without Impact	Multi-Lane

Detailed Truss Member Rating Results

Live Load: HS 20-44 (Design Truck)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand							
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF	
			Permit	Opr	Permit	Opr	Permit	Opr	Permit	Opr								
BC1	Lower-Chord	21.33		////		45.39 / 45.39	1.20		88.02			0.665		0.763	1.275			
		21.33		////		45.39 / 45.39	1.20		88.02			0.665		0.763	1.275			
		21.33		////		45.39 / 45.39			88.02			0.665		0.920	1.536			
		21.33		////		45.39 / 45.39			88.02			0.665		0.920	1.536			
BC2	Lower-Chord	21.33		////		45.39 / 45.39	1.20		93.75			0.665		0.836	1.396			
		21.33		////		45.39 / 45.39	1.20		93.75			0.665		0.836	1.396			
		21.33		////		45.39 / 45.39			93.75			0.665		1.007	1.682			
		21.33		////		45.39 / 45.39			93.75			0.665		1.007	1.682			
BC3	Lower-Chord	35.66		////		74.12 / 74.12	1.20		135.00			0.665		0.687	1.147			
		35.66		////		74.12 / 74.12	1.20		135.00			0.665		0.687	1.147			
		35.66		////		74.12 / 74.12			135.00			0.665		0.828	1.383			
		35.66		////		74.12 / 74.12			135.00			0.665		0.828	1.383			
BC4	Lower-Chord	42.42		////		84.15 / 84.15	1.20		157.50			0.665		0.699	1.167			
		42.42		////		84.15 / 84.15	1.20		157.50			0.665		0.699	1.167			
		42.42		////		84.15 / 84.15			157.50			0.665		0.842	1.406			
		42.42		////		84.15 / 84.15			157.50			0.665		0.842	1.406			
BC5	Lower-Chord	35.68		////		74.12 / 74.12	1.20		135.00			0.665		0.687	1.147			
		35.68		////		74.12 / 74.12	1.20		135.00			0.665		0.687	1.147			
		35.68		////		74.12 / 74.12			135.00			0.665		0.828	1.382			
		35.68		////		74.12 / 74.12			135.00			0.665		0.828	1.382			
BC6	Lower-Chord	21.37		////		45.39 / 45.39	1.20		93.75			0.665		0.835	1.395			
		21.37		////		45.39 / 45.39	1.20		93.75			0.665		0.835	1.395			
		21.37		////		45.39 / 45.39			93.75			0.665		1.006	1.680			
		21.37		////		45.39 / 45.39			93.75			0.665		1.006	1.680			
BC7	Lower-Chord	21.37		////		45.39 / 45.39	1.20		88.02			0.665		0.763	1.273			
		21.37		////		45.39 / 45.39	1.20		88.02			0.665		0.763	1.273			
		21.37		////		45.39 / 45.39			88.02			0.665		0.919	1.534			
		21.37		////		45.39 / 45.39			88.02			0.665		0.919	1.534			
TC2	Upper-Chord	-35.66		-74.12 / -74.12	1.20	////			-232.63			0.665		1.444	2.411			
		-35.66		-74.12 / -74.12	1.20	////			-232.63			0.665		1.444	2.411			
		-35.66		-74.12 / -74.12		////			-232.63			0.665		1.740	2.905			
		-35.66		-74.12 / -74.12		////			-232.63			0.665		1.740	2.905			
TC3	Upper-Chord	-42.89		-87.73 / -87.73	1.20	////			-232.63			0.665		1.158	1.934			
		-42.89		-87.73 / -87.73	1.20	////			-232.63			0.665		1.158	1.934			
		-42.89		-87.73 / -87.73		////			-232.63			0.665		1.396	2.331			
		-42.89		-87.73 / -87.73		////			-232.63			0.665		1.396	2.331			
TC4	Upper-Chord	-43.37		-87.40 / -87.18	1.20	////			-232.63			0.665		1.159	1.940			
		-43.37		-87.40 / -87.18	1.20	////			-232.63			0.665		1.159	1.940			
		-43.37		-87.40 / -87.18		////			-232.63			0.665		1.396	2.337			
		-43.37		-87.40 / -87.18		////			-232.63			0.665		1.396	2.337			
TC5	Upper-Chord	-42.90		-87.73 / -87.73	1.20	////			-232.63			0.665		1.158	1.934			
		-42.90		-87.73 / -87.73	1.20	////			-232.63			0.665		1.158	1.934			
		-42.90		-87.73 / -87.73		////			-232.63			0.665		1.396	2.331			
		-42.90		-87.73 / -87.73		////			-232.63			0.665		1.396	2.331			
TC6	Upper-Chord	-35.68		-74.12 / -74.12	1.20	////			-232.63			0.665		1.444	2.411			
		-35.68		-74.12 / -74.12	1.20	////			-232.63			0.665		1.444	2.411			
		-35.68		-74.12 / -74.12		////			-232.63			0.665		1.739	2.905			
		-35.68		-74.12 / -74.12		////			-232.63			0.665		1.739	2.905			
VW1	Vertical	7.47		////		39.06 / 39.06	1.30		41.34			0.665		0.431	0.720			

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.47		////		39.06 / 39.06 ///	1.30		41.34			0.665	0.431	0.720		
		7.47		////		39.06 / 39.06 ///			41.34			0.665	0.560	0.936		
		7.47		////		39.06 / 39.06 ///			41.34			0.665	0.560	0.936		
VW2	Vertical	-9.98		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.871	1.455		
		-9.98		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.871	1.455		
		-9.98		-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.085	1.811		
		-9.98		-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.085	1.811		
VW3	Vertical	-1.61		-24.81 / -24.54 ///	1.27	0.41 / 0.69 ///	1.27	-68.58	86.61			0.665	1.461	2.467		
		-1.61		-24.81 / -24.54 ///	1.27	0.41 / 0.69 ///	1.27	-68.58	86.61			0.665	1.461	2.467		
		-1.61		-24.81 / -24.54 ///		0.41 / 0.69 ///		-68.58	86.61			0.665	1.855	3.133		
		-1.61		-24.81 / -24.54 ///		0.41 / 0.69 ///		-68.58	86.61			0.665	1.855	3.133		
VW4	Vertical	-1.60		-24.80 / -24.52 ///	1.27	0.40 / 0.67 ///	1.27	-68.58	86.61			0.665	1.462	2.469		
		-1.60		-24.80 / -24.52 ///	1.27	0.40 / 0.67 ///	1.27	-68.58	86.61			0.665	1.462	2.469		
		-1.60		-24.80 / -24.52 ///		0.40 / 0.67 ///		-68.58	86.61			0.665	1.856	3.135		
		-1.60		-24.80 / -24.52 ///		0.40 / 0.67 ///		-68.58	86.61			0.665	1.856	3.135		
VW5	Vertical	-9.95		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.872	1.456		
		-9.95		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.872	1.456		
		-9.95		-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.085	1.812		
		-9.95		-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.085	1.812		
VW6R	Vertical	7.51		////		39.06 / 39.06 ///	1.30		44.44			0.665	0.473	0.790		
		7.51		////		39.06 / 39.06 ///	1.30		44.44			0.665	0.473	0.790		
		7.51		////		39.06 / 39.06 ///			44.44			0.665	0.615	1.026		
		7.51		////		39.06 / 39.06 ///			44.44			0.665	0.615	1.026		
TC1	Diagonal	-33.90		-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.254	2.095		
		-33.90		-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.254	2.095		
		-33.90		-72.14 / -72.14 ///		////	////	-201.57				0.665	1.511	2.524		
		-33.90		-72.14 / -72.14 ///		////	////	-201.57				0.665	1.511	2.524		
TC7	Diagonal	-33.96		-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.254	2.094		
		-33.96		-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.254	2.094		
		-33.96		-72.14 / -72.14 ///		////	////	-201.57				0.665	1.511	2.523		
		-33.96		-72.14 / -72.14 ///		////	////	-201.57				0.665	1.511	2.523		
DW1	Diagonal	22.77		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		84.38			0.665	0.526	0.879		
		22.77		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		84.38			0.665	0.526	0.879		
		22.77		-6.92 / -6.92 ///		58.90 / 58.90 ///			84.38			0.665	0.644	1.075		
		22.77		-6.92 / -6.92 ///		58.90 / 58.90 ///			84.38			0.665	0.644	1.075		
DW2	Diagonal	11.50		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		54.00			0.665	0.476	0.794		
		11.50		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		54.00			0.665	0.476	0.794		
		11.50		-19.20 / -19.20 ///		45.67 / 45.67 ///			54.00			0.665	0.592	0.988		
		11.50		-19.20 / -19.20 ///		45.67 / 45.67 ///			54.00			0.665	0.592	0.988		
DW3	Diagonal	0.76		-0.49 / -0.87 ///	1.27	31.92 / 31.57 ///	1.27		41.34			0.665	0.689	1.164		
		0.76		-0.49 / -0.87 ///	1.27	31.92 / 31.57 ///	1.27		41.34			0.665	0.689	1.164		
		0.76		-0.49 / -0.87 ///		31.92 / 31.57 ///			41.34			0.665	0.875	1.478		
		0.76		-0.49 / -0.87 ///		31.92 / 31.57 ///			41.34			0.665	0.875	1.478		
DW4	Diagonal	0.75		-0.49 / -0.86 ///	1.27	31.91 / 31.55 ///	1.27		41.34			0.665	0.690	1.165		
		0.75		-0.49 / -0.86 ///	1.27	31.91 / 31.55 ///	1.27		41.34			0.665	0.690	1.165		
		0.75		-0.49 / -0.86 ///		31.91 / 31.55 ///			41.34			0.665	0.876	1.479		
		0.75		-0.49 / -0.86 ///		31.91 / 31.55 ///			41.34			0.665	0.876	1.479		
DW5	Diagonal	11.47		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		54.00			0.665	0.476	0.795		
		11.47		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		54.00			0.665	0.476	0.795		
		11.47		-19.20 / -19.20 ///		45.67 / 45.67 ///			54.00			0.665	0.592	0.989		
		11.47		-19.20 / -19.20 ///		45.67 / 45.67 ///			54.00			0.665	0.592	0.989		
DW6	Diagonal	22.75		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		84.38			0.665	0.527	0.880		
		22.75		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		84.38			0.665	0.527	0.880		
		22.75		-6.92 / -6.92 ///		58.90 / 58.90 ///			84.38			0.665	0.644	1.076		
		22.75		-6.92 / -6.92 ///		58.90 / 58.90 ///			84.38			0.665	0.644	1.076		

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: HS 20-44 (Design Lane)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand								
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF		
			Permit	Opr	Permit	Opr	Permit	Opr	Permit	Opr									
BC1	Lower-Chord	21.33		////		38.91 / 38.91	////	1.20		88.02			0.665		0.890	1.487			
		21.33		////		38.91 / 38.91	////	1.20		88.02			0.665		0.890	1.487			
		21.33		////		38.91 / 38.91	////			88.02			0.665		1.073	1.791			
		21.33		////		38.91 / 38.91	////			88.02			0.665		1.073	1.791			
BC2	Lower-Chord	21.33		////		38.91 / 38.91	////	1.20		93.75			0.665		0.975	1.628			
		21.33		////		38.91 / 38.91	////	1.20		93.75			0.665		0.975	1.628			
		21.33		////		38.91 / 38.91	////			93.75			0.665		1.175	1.962			
		21.33		////		38.91 / 38.91	////			93.75			0.665		1.175	1.962			
BC3	Lower-Chord	35.66		////		64.85 / 64.85	////	1.20		135.00			0.665		0.785	1.311			
		35.66		////		64.85 / 64.85	////	1.20		135.00			0.665		0.785	1.311			
		35.66		////		64.85 / 64.85	////			135.00			0.665		0.946	1.580			
		35.66		////		64.85 / 64.85	////			135.00			0.665		0.946	1.580			
BC4	Lower-Chord	42.42		////		71.91 / 72.14	////	1.20		157.50			0.665		0.818	1.361			
		42.42		////		71.91 / 72.14	////	1.20		157.50			0.665		0.818	1.361			
		42.42		////		71.91 / 72.14	////			157.50			0.665		0.985	1.640			
		42.42		////		71.91 / 72.14	////			157.50			0.665		0.985	1.640			
BC5	Lower-Chord	35.68		////		64.85 / 64.85	////	1.20		135.00			0.665		0.785	1.311			
		35.68		////		64.85 / 64.85	////	1.20		135.00			0.665		0.785	1.311			
		35.68		////		64.85 / 64.85	////			135.00			0.665		0.946	1.580			
		35.68		////		64.85 / 64.85	////			135.00			0.665		0.946	1.580			
BC6	Lower-Chord	21.37		////		38.91 / 38.91	////	1.20		93.75			0.665		0.974	1.627			
		21.37		////		38.91 / 38.91	////	1.20		93.75			0.665		0.974	1.627			
		21.37		////		38.91 / 38.91	////			93.75			0.665		1.174	1.960			
		21.37		////		38.91 / 38.91	////			93.75			0.665		1.174	1.960			
BC7	Lower-Chord	21.37		////		38.91 / 38.91	////	1.20		88.02			0.665		0.889	1.485			
		21.37		////		38.91 / 38.91	////	1.20		88.02			0.665		0.889	1.485			
		21.37		////		38.91 / 38.91	////			88.02			0.665		1.072	1.790			
		21.37		////		38.91 / 38.91	////			88.02			0.665		1.072	1.790			
TC2	Upper-Chord	-35.66		-64.85 / -64.85	////	1.20		////		-232.63			0.665		1.650	2.756			
		-35.66		-64.85 / -64.85	////	1.20		////		-232.63			0.665		1.650	2.756			
		-35.66		-64.85 / -64.85	////			////		-232.63			0.665		1.988	3.320			
		-35.66		-64.85 / -64.85	////			////		-232.63			0.665		1.988	3.320			
TC3	Upper-Chord	-42.89		-77.83 / -77.83	////	1.20		////		-232.63			0.665		1.306	2.180			
		-42.89		-77.83 / -77.83	////	1.20		////		-232.63			0.665		1.306	2.180			
		-42.89		-77.83 / -77.83	////			////		-232.63			0.665		1.573	2.627			
		-42.89		-77.83 / -77.83	////			////		-232.63			0.665		1.573	2.627			
TC4	Upper-Chord	-43.37		-77.50 / -77.28	////	1.20		////		-232.63			0.665		1.307	2.188			
		-43.37		-77.50 / -77.28	////	1.20		////		-232.63			0.665		1.307	2.188			
		-43.37		-77.50 / -77.28	////			////		-232.63			0.665		1.574	2.636			
		-43.37		-77.50 / -77.28	////			////		-232.63			0.665		1.574	2.636			
TC5	Upper-Chord	-42.90		-77.83 / -77.83	////	1.20		////		-232.63			0.665		1.306	2.180			
		-42.90		-77.83 / -77.83	////	1.20		////		-232.63			0.665		1.306	2.180			
		-42.90		-77.83 / -77.83	////			////		-232.63			0.665		1.573	2.627			
		-42.90		-77.83 / -77.83	////			////		-232.63			0.665		1.573	2.627			
TC6	Upper-Chord	-35.68		-64.85 / -64.85	////	1.20		////		-232.63			0.665		1.650	2.755			
		-35.68		-64.85 / -64.85	////	1.20		////		-232.63			0.665		1.650	2.755			
		-35.68		-64.85 / -64.85	////			////		-232.63			0.665		1.988	3.320			
		-35.68		-64.85 / -64.85	////			////		-232.63			0.665		1.988	3.320			
VW1	Vertical	7.47		////				36.88 / 36.88	////	1.30		41.34		0.665		0.457	0.763		

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.47		////		36.88 / 36.88	1.30		41.34			0.665	0.457	0.763		
		7.47		////		36.88 / 36.88	1.30		41.34			0.665	0.594	0.991		
		7.47		////		36.88 / 36.88	1.30		41.34			0.665	0.594	0.991		
VW2	Vertical	-9.98		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.053	1.759		
		-9.98		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.053	1.759		
		-9.98		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.311	2.189		
		-9.98		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.311	2.189		
VW3	Vertical	-1.61		-18.89 / -18.62	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	1.919	3.251		
		-1.61		-18.89 / -18.62	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	1.919	3.251		
		-1.61		-18.89 / -18.62	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	2.437	4.128		
		-1.61		-18.89 / -18.62	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	2.437	4.128		
VW4	Vertical	-1.60		-18.88 / -18.61	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	1.920	3.254		
		-1.60		-18.88 / -18.61	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	1.920	3.254		
		-1.60		-18.88 / -18.61	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	2.438	4.132		
		-1.60		-18.88 / -18.61	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	2.438	4.132		
VW5	Vertical	-9.95		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.054	1.760		
		-9.95		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.054	1.760		
		-9.95		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.312	2.190		
		-9.95		-29.36 / -29.36	1.24	11.06 / 11.06	1.30	-68.58	86.61			0.665	1.312	2.190		
VW6R	Vertical	7.51		////		36.88 / 36.88	1.30		44.44			0.665	0.501	0.836		
		7.51		////		36.88 / 36.88	1.30		44.44			0.665	0.501	0.836		
		7.51		////		36.88 / 36.88	1.30		44.44			0.665	0.651	1.087		
		7.51		////		36.88 / 36.88	1.30		44.44			0.665	0.651	1.087		
TC1	Diagonal	-33.90		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.280	2.138		
		-33.90		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.280	2.138		
		-33.90		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.543	2.577		
		-33.90		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.543	2.577		
TC7	Diagonal	-33.96		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.280	2.137		
		-33.96		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.280	2.137		
		-33.96		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.542	2.575		
		-33.96		-70.67 / -70.67	1.20	////	1.20	-201.57				0.665	1.542	2.575		
DW1	Diagonal	22.77		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.584	0.976		
		22.77		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.584	0.976		
		22.77		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.715	1.193		
		22.77		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.715	1.193		
DW2	Diagonal	11.50		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.575	0.960		
		11.50		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.575	0.960		
		11.50		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.716	1.195		
		11.50		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.716	1.195		
DW3	Diagonal	0.76		////		24.30 / 23.95	1.27		41.34			0.665	0.905	1.534		
		0.76		////		24.30 / 23.95	1.27		41.34			0.665	0.905	1.534		
		0.76		////		24.30 / 23.95	1.27		41.34			0.665	1.149	1.947		
		0.76		////		24.30 / 23.95	1.27		41.34			0.665	1.149	1.947		
DW4	Diagonal	0.75		////		24.29 / 23.94	1.27		41.34			0.665	0.906	1.535		
		0.75		////		24.29 / 23.94	1.27		41.34			0.665	0.906	1.535		
		0.75		////		24.29 / 23.94	1.27		41.34			0.665	1.150	1.949		
		0.75		////		24.29 / 23.94	1.27		41.34			0.665	1.150	1.949		
DW5	Diagonal	11.47		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.575	0.961		
		11.47		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.575	0.961		
		11.47		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.716	1.196		
		11.47		-14.22 / -14.22	1.30	37.78 / 37.78	1.24		54.00			0.665	0.716	1.196		
DW6	Diagonal	22.75		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.585	0.976		
		22.75		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.585	0.976		
		22.75		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.715	1.194		
		22.75		-5.95 / -5.95	1.30	53.06 / 53.06	1.22		84.38			0.665	0.715	1.194		

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: Type 3 (Design Truck)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand							
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF	
			Permit	Opr	Permit	Opr	Permit	Opr	Permit	Opr								
BC1	Lower-Chord	21.33	////		32.16 / 32.16	///	1.20		88.02			0.665		1.077	1.799			
		21.33	////		32.16 / 32.16	///	1.20		88.02			0.665		1.077	1.799			
		21.33	////		32.16 / 32.16	///			88.02			0.665		1.298	2.167			
		21.33	////		32.16 / 32.16	///			88.02			0.665		1.298	2.167			
BC2	Lower-Chord	21.33	////		32.16 / 32.16	///	1.20		93.75			0.665		1.179	1.970			
		21.33	////		32.16 / 32.16	///	1.20		93.75			0.665		1.179	1.970			
		21.33	////		32.16 / 32.16	///			93.75			0.665		1.421	2.373			
		21.33	////		32.16 / 32.16	///			93.75			0.665		1.421	2.373			
BC3	Lower-Chord	35.66	////		52.76 / 52.76	///	1.20		135.00			0.665		0.965	1.612			
		35.66	////		52.76 / 52.76	///	1.20		135.00			0.665		0.965	1.612			
		35.66	////		52.76 / 52.76	///			135.00			0.665		1.163	1.942			
		35.66	////		52.76 / 52.76	///			135.00			0.665		1.163	1.942			
BC4	Lower-Chord	42.42	////		58.44 / 58.66	///	1.20		157.50			0.665		1.006	1.674			
		42.42	////		58.44 / 58.66	///	1.20		157.50			0.665		1.006	1.674			
		42.42	////		58.44 / 58.66	///			157.50			0.665		1.212	2.017			
		42.42	////		58.44 / 58.66	///			157.50			0.665		1.212	2.017			
BC5	Lower-Chord	35.68	////		52.76 / 52.76	///	1.20		135.00			0.665		0.965	1.611			
		35.68	////		52.76 / 52.76	///	1.20		135.00			0.665		0.965	1.611			
		35.68	////		52.76 / 52.76	///			135.00			0.665		1.163	1.942			
		35.68	////		52.76 / 52.76	///			135.00			0.665		1.163	1.942			
BC6	Lower-Chord	21.37	////		32.16 / 32.16	///	1.20		93.75			0.665		1.178	1.968			
		21.37	////		32.16 / 32.16	///	1.20		93.75			0.665		1.178	1.968			
		21.37	////		32.16 / 32.16	///			93.75			0.665		1.420	2.371			
		21.37	////		32.16 / 32.16	///			93.75			0.665		1.420	2.371			
BC7	Lower-Chord	21.37	////		32.16 / 32.16	///	1.20		88.02			0.665		1.076	1.797			
		21.37	////		32.16 / 32.16	///	1.20		88.02			0.665		1.076	1.797			
		21.37	////		32.16 / 32.16	///			88.02			0.665		1.297	2.165			
		21.37	////		32.16 / 32.16	///			88.02			0.665		1.297	2.165			
TC2	Upper-Chord	-35.66	-52.76 / -52.76	///	1.20	////			-232.63			0.665		2.028	3.387			
		-35.66	-52.76 / -52.76	///	1.20	////			-232.63			0.665		2.028	3.387			
		-35.66	-52.76 / -52.76	///		////			-232.63			0.665		2.444	4.081			
		-35.66	-52.76 / -52.76	///		////			-232.63			0.665		2.444	4.081			
TC3	Upper-Chord	-42.89	-62.64 / -62.64	///	1.20	////			-232.63			0.665		1.622	2.709			
		-42.89	-62.64 / -62.64	///	1.20	////			-232.63			0.665		1.622	2.709			
		-42.89	-62.64 / -62.64	///		////			-232.63			0.665		1.955	3.264			
		-42.89	-62.64 / -62.64	///		////			-232.63			0.665		1.955	3.264			
TC4	Upper-Chord	-43.37	-62.31 / -62.09	///	1.20	////			-232.63			0.665		1.625	2.723			
		-43.37	-62.31 / -62.09	///	1.20	////			-232.63			0.665		1.625	2.723			
		-43.37	-62.31 / -62.09	///		////			-232.63			0.665		1.958	3.281			
		-43.37	-62.31 / -62.09	///		////			-232.63			0.665		1.958	3.281			
TC5	Upper-Chord	-42.90	-62.64 / -62.64	///	1.20	////			-232.63			0.665		1.622	2.709			
		-42.90	-62.64 / -62.64	///	1.20	////			-232.63			0.665		1.622	2.709			
		-42.90	-62.64 / -62.64	///		////			-232.63			0.665		1.954	3.264			
		-42.90	-62.64 / -62.64	///		////			-232.63			0.665		1.954	3.264			
TC6	Upper-Chord	-35.68	-52.76 / -52.76	///	1.20	////			-232.63			0.665		2.028	3.387			
		-35.68	-52.76 / -52.76	///	1.20	////			-232.63			0.665		2.028	3.387			
		-35.68	-52.76 / -52.76	///		////			-232.63			0.665		2.444	4.081			
		-35.68	-52.76 / -52.76	///		////			-232.63			0.665		2.444	4.081			
VW1	Vertical	7.47	////			31.88 / 31.88	///	1.30		41.34			0.665		0.528	0.882		

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.47		////		31.88 / 31.88	1.30		41.34			0.665	0.528	0.882		
		7.47		////		31.88 / 31.88	1.30		41.34			0.665	0.687	1.147		
		7.47		////		31.88 / 31.88	1.30		41.34			0.665	0.687	1.147		
VW2	Vertical	-9.98		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665	1.215	2.030		
		-9.98		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665	1.215	2.030		
		-9.98		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665	1.513	2.526		
		-9.98		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665	1.513	2.526		
VW3	Vertical	-1.61		-17.90 / -17.63	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	2.025	3.434		
		-1.61		-17.90 / -17.63	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	2.025	3.434		
		-1.61		-17.90 / -17.63	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	2.571	4.360		
		-1.61		-17.90 / -17.63	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	2.571	4.360		
VW4	Vertical	-1.60		-17.89 / -17.62	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	2.026	3.437		
		-1.60		-17.89 / -17.62	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	2.026	3.437		
		-1.60		-17.89 / -17.62	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	2.573	4.364		
		-1.60		-17.89 / -17.62	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	2.573	4.364		
VW5	Vertical	-9.95		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665	1.216	2.031		
		-9.95		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665	1.216	2.031		
		-9.95		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665	1.514	2.528		
		-9.95		-25.45 / -25.45	1.24	11.16 / 11.16	1.30	-68.58	86.61			0.665	1.514	2.528		
VW6R	Vertical	7.51		////		31.88 / 31.88	1.30		44.44			0.665	0.579	0.967		
		7.51		////		31.88 / 31.88	1.30		44.44			0.665	0.579	0.967		
		7.51		////		31.88 / 31.88	1.30		44.44			0.665	0.753	1.257		
		7.51		////		31.88 / 31.88	1.30		44.44			0.665	0.753	1.257		
TC1	Diagonal	-33.90		-51.12 / -51.12	1.20	////	1.20	-201.57				0.665	1.770	2.956		
		-33.90		-51.12 / -51.12	1.20	////	1.20	-201.57				0.665	1.770	2.956		
		-33.90		-51.12 / -51.12	1.20	////	1.20	-201.57				0.665	2.133	3.562		
		-33.90		-51.12 / -51.12	1.20	////	1.20	-201.57				0.665	2.133	3.562		
TC7	Diagonal	-33.96		-51.12 / -51.12	1.20	////	1.20	-201.57				0.665	1.769	2.955		
		-33.96		-51.12 / -51.12	1.20	////	1.20	-201.57				0.665	1.769	2.955		
		-33.96		-51.12 / -51.12	1.20	////	1.20	-201.57				0.665	2.132	3.560		
		-33.96		-51.12 / -51.12	1.20	////	1.20	-201.57				0.665	2.132	3.560		
DW1	Diagonal	22.77		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665	0.739	1.235		
		22.77		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665	0.739	1.235		
		22.77		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665	0.904	1.510		
		22.77		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665	0.904	1.510		
DW2	Diagonal	11.50		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665	0.663	1.108		
		11.50		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665	0.663	1.108		
		11.50		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665	0.826	1.379		
		11.50		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665	0.826	1.379		
DW3	Diagonal	0.76		-0.33 / -0.33	1.27	23.03 / 22.68	1.27		41.34			0.665	0.955	1.620		
		0.76		-0.33 / -0.33	1.27	23.03 / 22.68	1.27		41.34			0.665	0.955	1.620		
		0.76		-0.33 / -0.33	1.27	23.03 / 22.68	1.27		41.34			0.665	1.213	2.057		
		0.76		-0.33 / -0.33	1.27	23.03 / 22.68	1.27		41.34			0.665	1.213	2.057		
DW4	Diagonal	0.75		-0.33 / -0.33	1.27	23.02 / 22.67	1.27		41.34			0.665	0.956	1.621		
		0.75		-0.33 / -0.33	1.27	23.02 / 22.67	1.27		41.34			0.665	0.956	1.621		
		0.75		-0.33 / -0.33	1.27	23.02 / 22.67	1.27		41.34			0.665	1.214	2.059		
		0.75		-0.33 / -0.33	1.27	23.02 / 22.67	1.27		41.34			0.665	1.214	2.059		
DW5	Diagonal	11.47		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665	0.664	1.109		
		11.47		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665	0.664	1.109		
		11.47		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665	0.826	1.380		
		11.47		-14.36 / -14.36	1.30	32.74 / 32.74	1.24		54.00			0.665	0.826	1.380		
DW6	Diagonal	22.75		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665	0.740	1.236		
		22.75		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665	0.740	1.236		
		22.75		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665	0.905	1.511		
		22.75		-5.51 / -5.51	1.30	41.93 / 41.93	1.22		84.38			0.665	0.905	1.511		

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: Type 3-3 (Design Truck)

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand							
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF	
			Permit	Opr	Permit	Opr	Permit	Opr	Permit	Opr								
BC1	Lower-Chord	21.33	////		42.50 / 42.50	///	1.20		88.02			0.665		0.815	1.361			
		21.33	////		42.50 / 42.50	///	1.20		88.02			0.665		0.815	1.361			
		21.33	////		42.50 / 42.50	///			88.02			0.665		0.982	1.640			
		21.33	////		42.50 / 42.50	///			88.02			0.665		0.982	1.640			
BC2	Lower-Chord	21.33	////		42.50 / 42.50	///	1.20		93.75			0.665		0.892	1.490			
		21.33	////		42.50 / 42.50	///	1.20		93.75			0.665		0.892	1.490			
		21.33	////		42.50 / 42.50	///			93.75			0.665		1.075	1.796			
		21.33	////		42.50 / 42.50	///			93.75			0.665		1.075	1.796			
BC3	Lower-Chord	35.66	////		68.19 / 68.19	///	1.20		135.00			0.665		0.747	1.247			
		35.66	////		68.19 / 68.19	///	1.20		135.00			0.665		0.747	1.247			
		35.66	////		68.19 / 68.19	///			135.00			0.665		0.900	1.503			
		35.66	////		68.19 / 68.19	///			135.00			0.665		0.900	1.503			
BC4	Lower-Chord	42.42	////		78.80 / 78.80	///	1.20		157.50			0.665		0.746	1.246			
		42.42	////		78.80 / 78.80	///	1.20		157.50			0.665		0.746	1.246			
		42.42	////		78.80 / 78.80	///			157.50			0.665		0.899	1.502			
		42.42	////		78.80 / 78.80	///			157.50			0.665		0.899	1.502			
BC5	Lower-Chord	35.68	////		68.19 / 68.19	///	1.20		135.00			0.665		0.747	1.247			
		35.68	////		68.19 / 68.19	///	1.20		135.00			0.665		0.747	1.247			
		35.68	////		68.19 / 68.19	///			135.00			0.665		0.900	1.502			
		35.68	////		68.19 / 68.19	///			135.00			0.665		0.900	1.502			
BC6	Lower-Chord	21.37	////		42.50 / 42.50	///	1.20		93.75			0.665		0.892	1.489			
		21.37	////		42.50 / 42.50	///	1.20		93.75			0.665		0.892	1.489			
		21.37	////		42.50 / 42.50	///			93.75			0.665		1.074	1.794			
		21.37	////		42.50 / 42.50	///			93.75			0.665		1.074	1.794			
BC7	Lower-Chord	21.37	////		42.50 / 42.50	///	1.20		88.02			0.665		0.814	1.360			
		21.37	////		42.50 / 42.50	///	1.20		88.02			0.665		0.814	1.360			
		21.37	////		42.50 / 42.50	///			88.02			0.665		0.981	1.638			
		21.37	////		42.50 / 42.50	///			88.02			0.665		0.981	1.638			
TC2	Upper-Chord	-35.66	-68.19 / -68.19	///	1.20	////			-232.63			0.665		1.569	2.621			
		-35.66	-68.19 / -68.19	///	1.20	////			-232.63			0.665		1.569	2.621			
		-35.66	-68.19 / -68.19	///		////			-232.63			0.665		1.891	3.158			
		-35.66	-68.19 / -68.19	///		////			-232.63			0.665		1.891	3.158			
TC3	Upper-Chord	-42.89	-80.65 / -80.65	///	1.20	////			-232.63			0.665		1.260	2.104			
		-42.89	-80.65 / -80.65	///	1.20	////			-232.63			0.665		1.260	2.104			
		-42.89	-80.65 / -80.65	///		////			-232.63			0.665		1.518	2.535			
		-42.89	-80.65 / -80.65	///		////			-232.63			0.665		1.518	2.535			
TC4	Upper-Chord	-43.37	-81.01 / -81.01	///	1.20	////			-232.63			0.665		1.250	2.087			
		-43.37	-81.01 / -81.01	///	1.20	////			-232.63			0.665		1.250	2.087			
		-43.37	-81.01 / -81.01	///		////			-232.63			0.665		1.506	2.515			
		-43.37	-81.01 / -81.01	///		////			-232.63			0.665		1.506	2.515			
TC5	Upper-Chord	-42.90	-80.65 / -80.65	///	1.20	////			-232.63			0.665		1.260	2.104			
		-42.90	-80.65 / -80.65	///	1.20	////			-232.63			0.665		1.260	2.104			
		-42.90	-80.65 / -80.65	///		////			-232.63			0.665		1.518	2.535			
		-42.90	-80.65 / -80.65	///		////			-232.63			0.665		1.518	2.535			
TC6	Upper-Chord	-35.68	-68.19 / -68.19	///	1.20	////			-232.63			0.665		1.569	2.620			
		-35.68	-68.19 / -68.19	///	1.20	////			-232.63			0.665		1.569	2.620			
		-35.68	-68.19 / -68.19	///		////			-232.63			0.665		1.891	3.157			
		-35.68	-68.19 / -68.19	///		////			-232.63			0.665		1.891	3.157			
VW1	Vertical	7.47	////		25.65 / 25.65	///	1.30		41.34			0.665		0.657	1.097			

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.47		////		25.65 / 25.65	1.30		41.34			0.665	0.657	1.097		
		7.47		////		25.65 / 25.65	1.30		41.34			0.665	0.854	1.425		
		7.47		////		25.65 / 25.65	1.30		41.34			0.665	0.854	1.425		
VW2	Vertical	-9.98		-29.65 / -29.65	1.24	9.41 / 9.41	1.30	-68.58	86.61			0.665		1.043	1.742	
		-9.98		-29.65 / -29.65	1.24	9.41 / 9.41	1.30	-68.58	86.61			0.665		1.043	1.742	
		-9.98		-29.65 / -29.65	1.24	9.41 / 9.41	1.30	-68.58	86.61			0.665		1.298	2.168	
		-9.98		-29.65 / -29.65	1.24	9.41 / 9.41	1.30	-68.58	86.61			0.665		1.298	2.168	
VW3	Vertical	-1.61		-18.12 / -17.85	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665		2.001	3.392	
		-1.61		-18.12 / -17.85	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665		2.001	3.392	
		-1.61		-18.12 / -17.85	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665		2.540	4.307	
		-1.61		-18.12 / -17.85	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665		2.540	4.307	
VW4	Vertical	-1.60		-18.11 / -17.84	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665		2.002	3.395	
		-1.60		-18.11 / -17.84	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665		2.002	3.395	
		-1.60		-18.11 / -17.84	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665		2.542	4.310	
		-1.60		-18.11 / -17.84	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665		2.542	4.310	
VW5	Vertical	-9.95		-29.65 / -29.65	1.24	9.41 / 9.41	1.30	-68.58	86.61			0.665		1.044	1.743	
		-9.95		-29.65 / -29.65	1.24	9.41 / 9.41	1.30	-68.58	86.61			0.665		1.044	1.743	
		-9.95		-29.65 / -29.65	1.24	9.41 / 9.41	1.30	-68.58	86.61			0.665		1.299	2.169	
		-9.95		-29.65 / -29.65	1.24	9.41 / 9.41	1.30	-68.58	86.61			0.665		1.299	2.169	
VW6R	Vertical	7.51		////		25.65 / 25.65	1.30		44.44			0.665		0.720	1.202	
		7.51		////		25.65 / 25.65	1.30		44.44			0.665		0.720	1.202	
		7.51		////		25.65 / 25.65	1.30		44.44			0.665		0.936	1.563	
		7.51		////		25.65 / 25.65	1.30		44.44			0.665		0.936	1.563	
TC1	Diagonal	-33.90		-67.55 / -67.55	1.20	////	////	-201.57				0.665		1.340	2.237	
		-33.90		-67.55 / -67.55	1.20	////	////	-201.57				0.665		1.340	2.237	
		-33.90		-67.55 / -67.55	1.20	////	////	-201.57				0.665		1.614	2.695	
		-33.90		-67.55 / -67.55	1.20	////	////	-201.57				0.665		1.614	2.695	
TC7	Diagonal	-33.96		-67.55 / -67.55	1.20	////	////	-201.57				0.665		1.339	2.236	
		-33.96		-67.55 / -67.55	1.20	////	////	-201.57				0.665		1.339	2.236	
		-33.96		-67.55 / -67.55	1.20	////	////	-201.57				0.665		1.613	2.694	
		-33.96		-67.55 / -67.55	1.20	////	////	-201.57				0.665		1.613	2.694	
DW1	Diagonal	22.77		-4.54 / -4.54	1.30	52.85 / 52.85	1.22		84.38			0.665		0.587	0.980	
		22.77		-4.54 / -4.54	1.30	52.85 / 52.85	1.22		84.38			0.665		0.587	0.980	
		22.77		-4.54 / -4.54	1.30	52.85 / 52.85	1.22		84.38			0.665		0.717	1.198	
		22.77		-4.54 / -4.54	1.30	52.85 / 52.85	1.22		84.38			0.665		0.717	1.198	
DW2	Diagonal	11.50		-12.11 / -12.11	1.30	38.14 / 38.14	1.24		54.00			0.665		0.569	0.951	
		11.50		-12.11 / -12.11	1.30	38.14 / 38.14	1.24		54.00			0.665		0.569	0.951	
		11.50		-12.11 / -12.11	1.30	38.14 / 38.14	1.24		54.00			0.665		0.709	1.183	
		11.50		-12.11 / -12.11	1.30	38.14 / 38.14	1.24		54.00			0.665		0.709	1.183	
DW3	Diagonal	0.76		-0.48 / -0.88	1.27	23.31 / 22.96	1.27		41.34			0.665		0.944	1.600	
		0.76		-0.48 / -0.88	1.27	23.31 / 22.96	1.27		41.34			0.665		0.944	1.600	
		0.76		-0.48 / -0.88	1.27	23.31 / 22.96	1.27		41.34			0.665		1.198	2.031	
		0.76		-0.48 / -0.88	1.27	23.31 / 22.96	1.27		41.34			0.665		1.198	2.031	
DW4	Diagonal	0.75		-0.48 / -0.74	1.27	23.30 / 22.95	1.27		41.34			0.665		0.944	1.601	
		0.75		-0.48 / -0.74	1.27	23.30 / 22.95	1.27		41.34			0.665		0.944	1.601	
		0.75		-0.48 / -0.74	1.27	23.30 / 22.95	1.27		41.34			0.665		1.199	2.034	
		0.75		-0.48 / -0.74	1.27	23.30 / 22.95	1.27		41.34			0.665		1.199	2.034	
DW5	Diagonal	11.47		-12.11 / -12.11	1.30	38.14 / 38.14	1.24		54.00			0.665		0.570	0.952	
		11.47		-12.11 / -12.11	1.30	38.14 / 38.14	1.24		54.00			0.665		0.570	0.952	
		11.47		-12.11 / -12.11	1.30	38.14 / 38.14	1.24		54.00			0.665		0.709	1.185	
		11.47		-12.11 / -12.11	1.30	38.14 / 38.14	1.24		54.00			0.665		0.709	1.185	
DW6	Diagonal	22.75		-4.54 / -4.54	1.30	52.85 / 52.85	1.22		84.38			0.665		0.587	0.980	
		22.75		-4.54 / -4.54	1.30	52.85 / 52.85	1.22		84.38			0.665		0.587	0.980	
		22.75		-4.54 / -4.54	1.30	52.85 / 52.85	1.22		84.38			0.665		0.718	1.199	
		22.75		-4.54 / -4.54	1.30	52.85 / 52.85	1.22		84.38			0.665		0.718	1.199	

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: Type 3S2 (Design Truck)

LL Scale Factor = 1.00
Adjacent Vehicle LL Factor = 0.00

Inventory:

A1 = 1.30 , A2 = 2.17

Operating:

A1 = 1.30 , A2 = 1.30

Note: Rating factor is outputted as 99.00 when it is greater than 99

Member	Truss Element	DL Force (kip)	LL Force				Capacity				Adj Veh Demand							
			Comp. Inv / Opr / Legal		Tens. Inv / Opr / Legal		IF	Comp. (kip)	Tens. (kip)	Comp. (kip)	Tens. (kip)	One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF
			Permit	Opr	Permit	Opr												
BC1	Lower-Chord	21.33		////		40.84 / 40.84	1.20		88.02			0.665		0.848	1.416			
		21.33		////		40.84 / 40.84	1.20		88.02			0.665		0.848	1.416			
		21.33		////		40.84 / 40.84			88.02			0.665		1.022	1.707			
		21.33		////		40.84 / 40.84			88.02			0.665		1.022	1.707			
BC2	Lower-Chord	21.33		////		40.84 / 40.84	1.20		93.75			0.665		0.929	1.551			
		21.33		////		40.84 / 40.84	1.20		93.75			0.665		0.929	1.551			
		21.33		////		40.84 / 40.84			93.75			0.665		1.119	1.869			
		21.33		////		40.84 / 40.84			93.75			0.665		1.119	1.869			
BC3	Lower-Chord	35.66		////		66.87 / 66.87	1.20		135.00			0.665		0.762	1.272			
		35.66		////		66.87 / 66.87	1.20		135.00			0.665		0.762	1.272			
		35.66		////		66.87 / 66.87			135.00			0.665		0.918	1.532			
		35.66		////		66.87 / 66.87			135.00			0.665		0.918	1.532			
BC4	Lower-Chord	42.42		////		75.79 / 76.02	1.20		157.50			0.665		0.776	1.292			
		42.42		////		75.79 / 76.02	1.20		157.50			0.665		0.776	1.292			
		42.42		////		75.79 / 76.02			157.50			0.665		0.935	1.557			
		42.42		////		75.79 / 76.02			157.50			0.665		0.935	1.557			
BC5	Lower-Chord	35.68		////		66.87 / 66.87	1.20		135.00			0.665		0.761	1.271			
		35.68		////		66.87 / 66.87	1.20		135.00			0.665		0.761	1.271			
		35.68		////		66.87 / 66.87			135.00			0.665		0.917	1.532			
		35.68		////		66.87 / 66.87			135.00			0.665		0.917	1.532			
BC6	Lower-Chord	21.37		////		40.84 / 40.84	1.20		93.75			0.665		0.928	1.550			
		21.37		////		40.84 / 40.84	1.20		93.75			0.665		0.928	1.550			
		21.37		////		40.84 / 40.84			93.75			0.665		1.118	1.867			
		21.37		////		40.84 / 40.84			93.75			0.665		1.118	1.867			
BC7	Lower-Chord	21.37		////		40.84 / 40.84	1.20		88.02			0.665		0.847	1.415			
		21.37		////		40.84 / 40.84	1.20		88.02			0.665		0.847	1.415			
		21.37		////		40.84 / 40.84			88.02			0.665		1.021	1.705			
		21.37		////		40.84 / 40.84			88.02			0.665		1.021	1.705			
TC2	Upper-Chord	-35.66		-66.87 / -66.87	1.20	////		-232.63				0.665		1.600	2.673			
		-35.66		-66.87 / -66.87	1.20	////		-232.63				0.665		1.600	2.673			
		-35.66		-66.87 / -66.87		////		-232.63				0.665		1.928	3.220			
		-35.66		-66.87 / -66.87		////		-232.63				0.665		1.928	3.220			
TC3	Upper-Chord	-42.89		-78.97 / -78.97	1.20	////		-232.63				0.665		1.287	2.149			
		-42.89		-78.97 / -78.97	1.20	////		-232.63				0.665		1.287	2.149			
		-42.89		-78.97 / -78.97		////		-232.63				0.665		1.550	2.589			
		-42.89		-78.97 / -78.97		////		-232.63				0.665		1.550	2.589			
TC4	Upper-Chord	-43.37		-78.64 / -78.42	1.20	////		-232.63				0.665		1.288	2.156			
		-43.37		-78.64 / -78.42	1.20	////		-232.63				0.665		1.288	2.156			
		-43.37		-78.64 / -78.42		////		-232.63				0.665		1.551	2.598			
		-43.37		-78.64 / -78.42		////		-232.63				0.665		1.551	2.598			
TC5	Upper-Chord	-42.90		-78.97 / -78.97	1.20	////		-232.63				0.665		1.287	2.149			
		-42.90		-78.97 / -78.97	1.20	////		-232.63				0.665		1.287	2.149			
		-42.90		-78.97 / -78.97		////		-232.63				0.665		1.550	2.589			
		-42.90		-78.97 / -78.97		////		-232.63				0.665		1.550	2.589			
TC6	Upper-Chord	-35.68		-66.87 / -66.87	1.20	////		-232.63				0.665		1.600	2.672			
		-35.68		-66.87 / -66.87	1.20	////		-232.63				0.665		1.600	2.672			
		-35.68		-66.87 / -66.87		////		-232.63				0.665		1.928	3.220			
		-35.68		-66.87 / -66.87		////		-232.63				0.665		1.928	3.220			
VW1	Vertical	7.47		////		30.88 / 30.88	1.30		41.34			0.665		0.545	0.911			

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.47		////		30.88 / 30.88	1.30		41.34			0.665	0.545	0.911			
		7.47		////		30.88 / 30.88	1.30		41.34			0.665	0.709	1.184			
		7.47		////		30.88 / 30.88	1.30		41.34			0.665	0.709	1.184			
VW2	Vertical	-9.98		-29.88 / -29.88	1.24	9.90 / 9.90	1.30	-68.58	86.61			0.665	1.035	1.728			
		-9.98		-29.88 / -29.88	1.24	9.90 / 9.90	1.30	-68.58	86.61			0.665	1.035	1.728			
		-9.98		-29.88 / -29.88	1.24	9.90 / 9.90	1.30	-68.58	86.61			0.665	1.288	2.151			
		-9.98		-29.88 / -29.88	1.24	9.90 / 9.90	1.30	-68.58	86.61			0.665	1.288	2.151			
VW3	Vertical	-1.61		-19.19 / -18.92	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	1.888	3.199			
		-1.61		-19.19 / -18.92	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	1.888	3.199			
		-1.61		-19.19 / -18.92	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	2.398	4.062			
		-1.61		-19.19 / -18.92	1.27	0.41 / 0.69	1.27	-68.58	86.61			0.665	2.398	4.062			
VW4	Vertical	-1.60		-19.19 / -18.91	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	1.890	3.201			
		-1.60		-19.19 / -18.91	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	1.890	3.201			
		-1.60		-19.19 / -18.91	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	2.399	4.065			
		-1.60		-19.19 / -18.91	1.27	0.40 / 0.67	1.27	-68.58	86.61			0.665	2.399	4.065			
VW5	Vertical	-9.95		-29.88 / -29.88	1.24	9.90 / 9.90	1.30	-68.58	86.61			0.665	1.035	1.729			
		-9.95		-29.88 / -29.88	1.24	9.90 / 9.90	1.30	-68.58	86.61			0.665	1.035	1.729			
		-9.95		-29.88 / -29.88	1.24	9.90 / 9.90	1.30	-68.58	86.61			0.665	1.289	2.152			
		-9.95		-29.88 / -29.88	1.24	9.90 / 9.90	1.30	-68.58	86.61			0.665	1.289	2.152			
VW6R	Vertical	7.51		////		30.88 / 30.88	1.30		44.44			0.665	0.598	0.999			
		7.51		////		30.88 / 30.88	1.30		44.44			0.665	0.598	0.999			
		7.51		////		30.88 / 30.88	1.30		44.44			0.665	0.777	1.298			
		7.51		////		30.88 / 30.88	1.30		44.44			0.665	0.777	1.298			
TC1	Diagonal	-33.90		-64.91 / -64.91	1.20	////	1.20	-201.57				0.665	1.394	2.328			
		-33.90		-64.91 / -64.91	1.20	////	1.20	-201.57				0.665	1.394	2.328			
		-33.90		-64.91 / -64.91	1.20	////	1.20	-201.57				0.665	1.680	2.805			
		-33.90		-64.91 / -64.91	1.20	////	1.20	-201.57				0.665	1.680	2.805			
TC7	Diagonal	-33.96		-64.91 / -64.91	1.20	////	1.20	-201.57				0.665	1.393	2.327			
		-33.96		-64.91 / -64.91	1.20	////	1.20	-201.57				0.665	1.393	2.327			
		-33.96		-64.91 / -64.91	1.20	////	1.20	-201.57				0.665	1.679	2.803			
		-33.96		-64.91 / -64.91	1.20	////	1.20	-201.57				0.665	1.679	2.803			
DW1	Diagonal	22.77		-5.03 / -5.03	1.30	51.68 / 51.68	1.22		84.38			0.665	0.600	1.002			
		22.77		-5.03 / -5.03	1.30	51.68 / 51.68	1.22		84.38			0.665	0.600	1.002			
		22.77		-5.03 / -5.03	1.30	51.68 / 51.68	1.22		84.38			0.665	0.734	1.225			
		22.77		-5.03 / -5.03	1.30	51.68 / 51.68	1.22		84.38			0.665	0.734	1.225			
DW2	Diagonal	11.50		-12.74 / -12.74	1.30	38.45 / 38.45	1.24		54.00			0.665	0.565	0.943			
		11.50		-12.74 / -12.74	1.30	38.45 / 38.45	1.24		54.00			0.665	0.565	0.943			
		11.50		-12.74 / -12.74	1.30	38.45 / 38.45	1.24		54.00			0.665	0.703	1.174			
		11.50		-12.74 / -12.74	1.30	38.45 / 38.45	1.24		54.00			0.665	0.703	1.174			
DW3	Diagonal	0.76		-0.32 / -0.78	1.27	24.69 / 24.35	1.27		41.34			0.665	0.891	1.509			
		0.76		-0.32 / -0.78	1.27	24.69 / 24.35	1.27		41.34			0.665	0.891	1.509			
		0.76		-0.32 / -0.78	1.27	24.69 / 24.35	1.27		41.34			0.665	1.131	1.916			
		0.76		-0.32 / -0.78	1.27	24.69 / 24.35	1.27		41.34			0.665	1.131	1.916			
DW4	Diagonal	0.75		-0.32 / -0.78	1.27	24.68 / 24.33	1.27		41.34			0.665	0.891	1.510			
		0.75		-0.32 / -0.78	1.27	24.68 / 24.33	1.27		41.34			0.665	0.891	1.510			
		0.75		-0.32 / -0.78	1.27	24.68 / 24.33	1.27		41.34			0.665	1.132	1.918			
		0.75		-0.32 / -0.78	1.27	24.68 / 24.33	1.27		41.34			0.665	1.132	1.918			
DW5	Diagonal	11.47		-12.74 / -12.74	1.30	38.45 / 38.45	1.24		54.00			0.665	0.565	0.944			
		11.47		-12.74 / -12.74	1.30	38.45 / 38.45	1.24		54.00			0.665	0.565	0.944			
		11.47		-12.74 / -12.74	1.30	38.45 / 38.45	1.24		54.00			0.665	0.704	1.175			
		11.47		-12.74 / -12.74	1.30	38.45 / 38.45	1.24		54.00			0.665	0.704	1.175			
DW6	Diagonal	22.75		-5.03 / -5.03	1.30	51.68 / 51.68	1.22		84.38			0.665	0.600	1.002			
		22.75		-5.03 / -5.03	1.30	51.68 / 51.68	1.22		84.38			0.665	0.600	1.002			
		22.75		-5.03 / -5.03	1.30	51.68 / 51.68	1.22		84.38			0.665	0.734	1.226			
		22.75		-5.03 / -5.03	1.30	51.68 / 51.68	1.22		84.38			0.665	0.734	1.226			

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500



PROJECT MDT Bridge Rating SEI 13-035

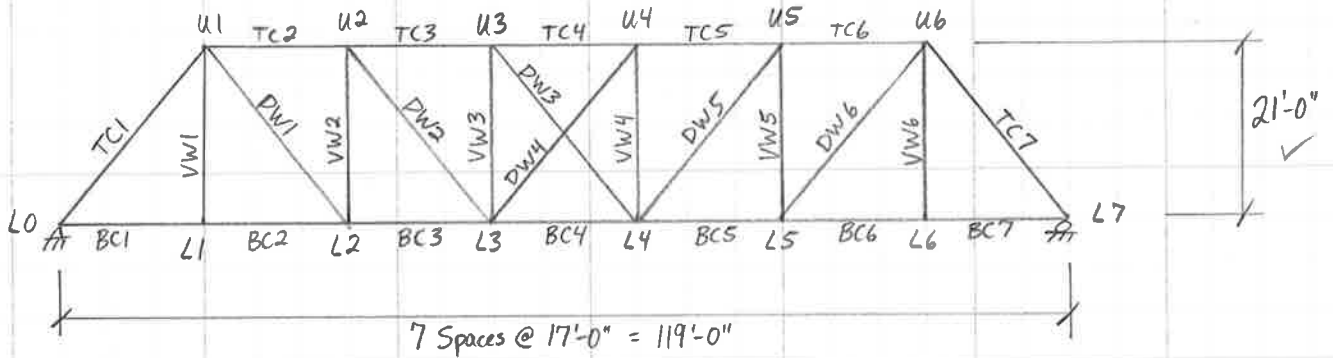
SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 12/30/13

CHECKED BY RDL DATE 12-1-14

L 15672000+02001 Swan River (Truss)

Establish truss geometry and naming convention



Panel Point Coordinates ✓

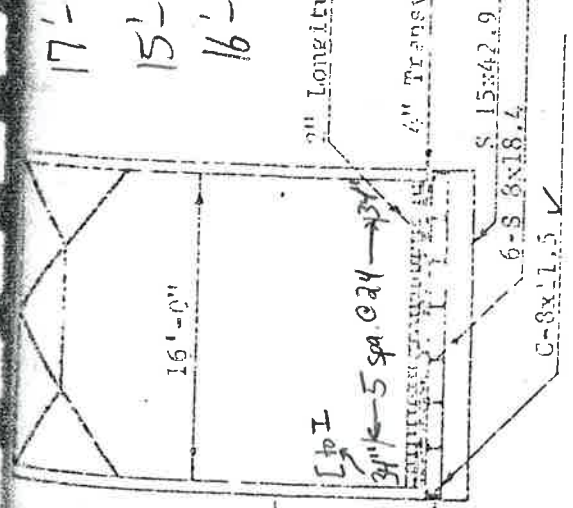
L0	(0,0)	U1	(17,21)
L1	(17,0)	U2	(34,21)
L2	(34,0)	U3	(51,21)
L3	(51,0)	U4	(68,21)
L4	(68,0)	U5	(85,21)
L5	(85,0)	U6	(102,21)
L6	(102,0)		
L7	(119,0)		



17'-11 3/4" : out-to-out latticed VW
 15'-7 1/2" : Face of rail - Face of rail
 16'-1" : deck width

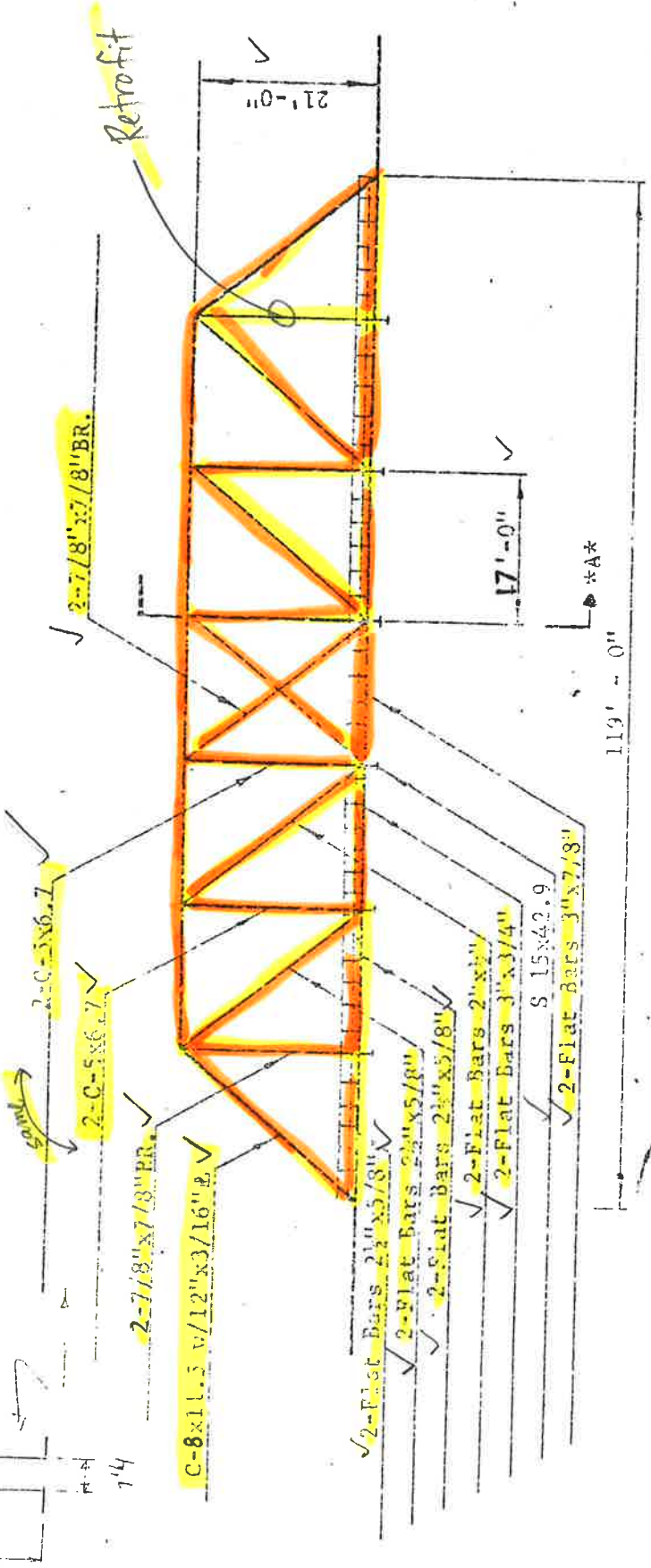
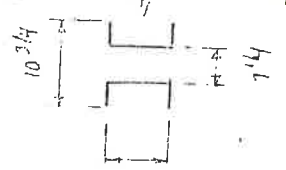
3" Longitudinal Planks 9 1/2 x 2 1/2 running planks
 4" Transverse Members 4 x (9 1/2 -> 12) deck planks

SEC. 36, T27N, R20W



2-C-5 x 6.7
 2-Flat Bars 3" x 3/4"

SECTION ***
 Scale: 1" = 10'-0"



2-7/8" x 7/8" BR.

2-C-5 x 6.7

2-7/8" x 7/8" BR.

C-8 x 11.5 v 12" x 3/16" PL

2-Flat Bars 2 1/4" x 5/8"

2-Flat Bars 2 1/4" x 5/8"

2-Flat Bars 2" x 1/2"

2-Flat Bars 3" x 1/4"

S 15 x 42.9

2-Flat Bars 3" x 7/8"

119'-0" *A*

ELEVATION VIEW

Scale: 1" = 20'-0"



L15672000+02001

PROJECT MDT Bridge Rating SEI 13-035

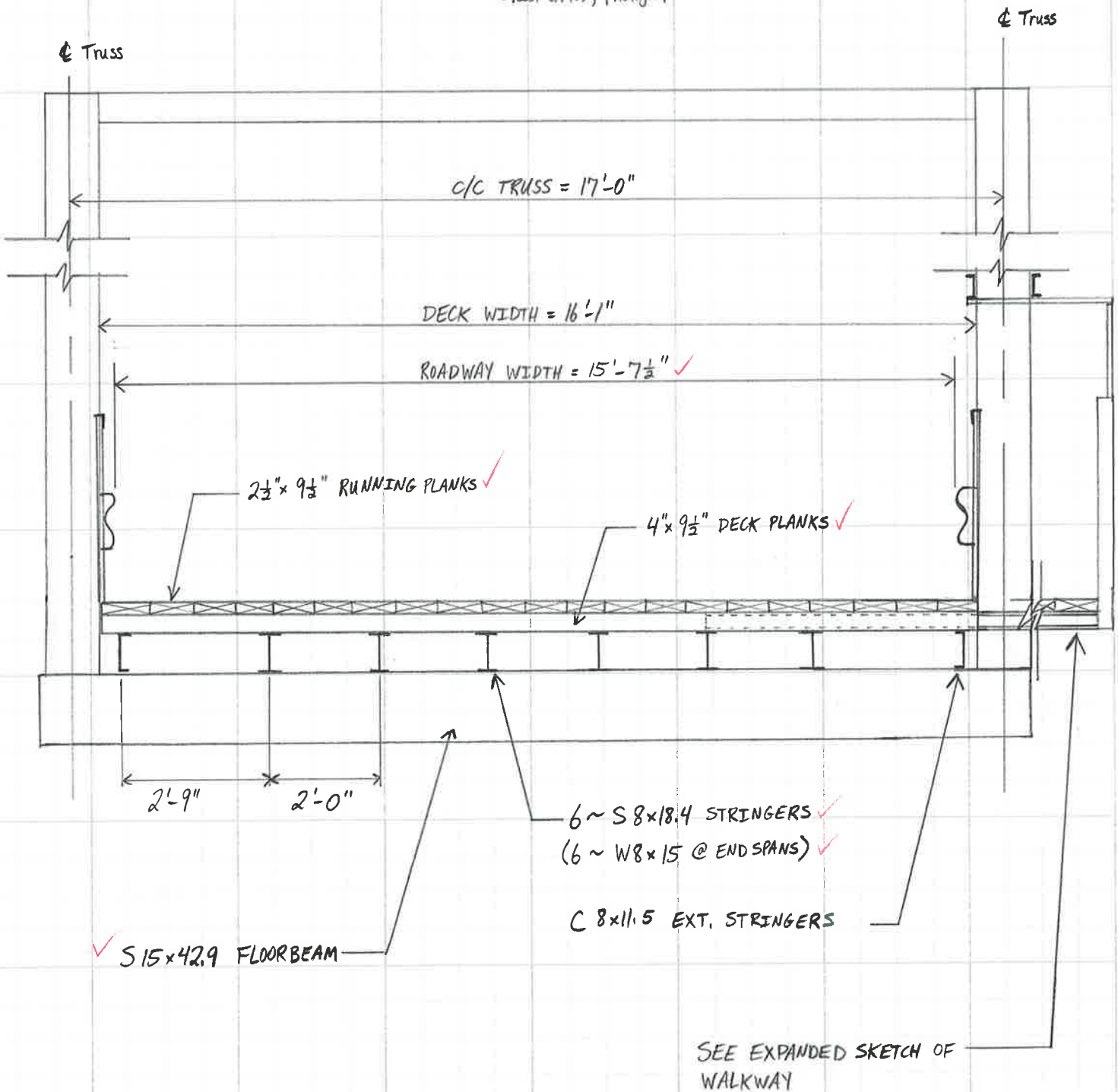
SHEET NO. _____ OF _____

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Bridge Cross Section Composite Sketch

— Info from SEI (KLJ) inspection Oct. 2013, Structure Inventory Sheet (1978), Photographs



Scale : $\frac{3}{8}" = 1'-0"$



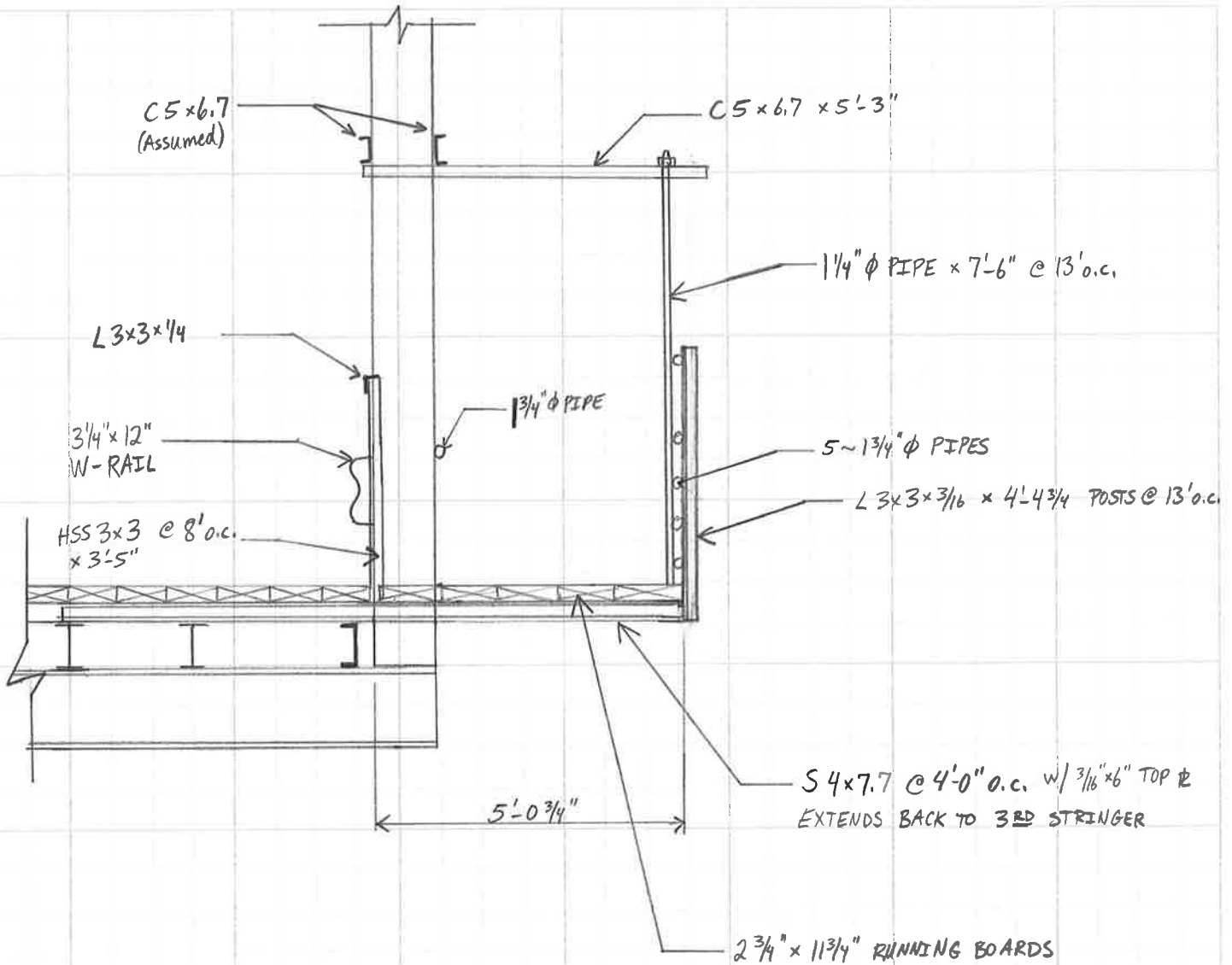
L 15672000+02.001

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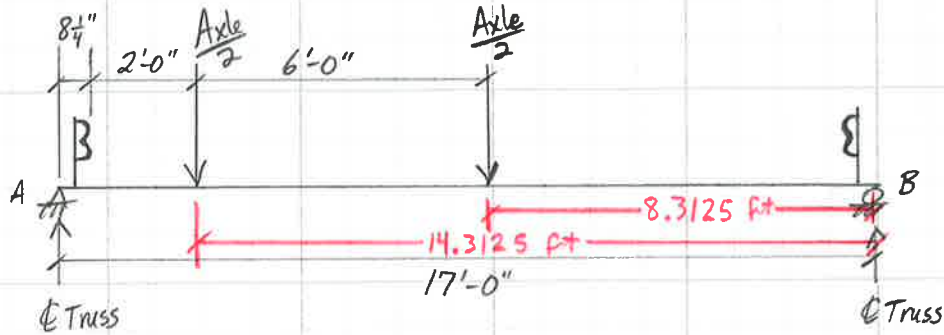


Walkway on SE side of Bridge



L15672000+02001

Calculate LLDF for Main Truss



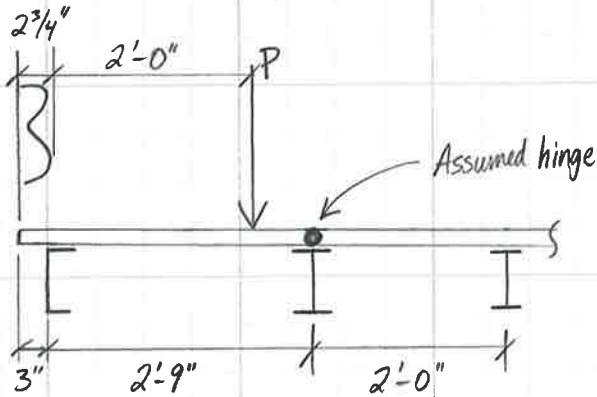
Z AXIS @ B

$$\sum M_B = 0 = R_A(17ft) - \frac{A}{2}(14.3125 ft) - \frac{A}{2}(8.3125 ft)$$

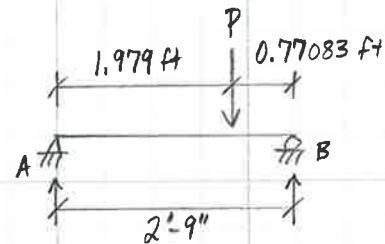
$$R_A = 0.6654 \text{ Axle}$$

$$\therefore \underline{\underline{LLDF = 0.6654}} \checkmark$$

Calculate Exterior Stringer LLDF - per AASHTO 3.23.2.3.1



⇒



$$\sum M_B = 0 = R_A(2.75 ft) - P(0.77083 ft)$$

$$R_A = 0.2803 P$$

$$\therefore \underline{\underline{LLDF = 0.2803}} \checkmark$$

Calculate Deflection LLDF

$$DF = \frac{\# \text{ wheel lines}}{\# \text{ stringers}} = \frac{2}{8} = 0.25 \checkmark$$

PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 1/2/14CHECKED BY RDL DATE 12-1-14

L15672000+02001

Exterior Stringer Size

The exterior stringer is a C 8x11.5. Virtis does not support channel stringers. Therefore, a comparable S or W shape must be selected.

C 8x11.5 ✓

$$A = 3.37 \text{ in}^2$$

$$A_w = 0.22 \text{ in} \times 8 \text{ in} = 1.76 \text{ in}^2$$

$$I_x = 32.5 \text{ in}^4$$

$$S_x = 8.14 \text{ in}^3$$

$$Z_x = 9.63 \text{ in}^3$$

S 6x17.25

$$A = 5.05 \text{ in}^2$$

$$A_w = 0.465 \text{ in} \times 6 \text{ in} = 2.79 \text{ in}^2$$

$$I_x = 26.2 \text{ in}^4$$

$$S_x = 8.74 \text{ in}^3$$

$$Z_x = 10.5 \text{ in}^3$$

S 6x12.5

$$A = 3.66 \text{ in}^2$$

$$A_w = 0.232 \text{ in} \times 6 \text{ in} = 1.39 \text{ in}^2$$

$$I_x = 22.0 \text{ in}^4$$

$$S_x = 7.34 \text{ in}^3$$

$$Z_x = 8.45 \text{ in}^3$$

* Select the S 6x12.5 for the exterior stringer in the Virtis model.
The cross sectional properties are slightly conservative compared to the C 8x11.5.

PROJECT MDT Bridge Rating SEI 13-035

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CALCULATED BY BBL DATE 12/30/13CHECKED BY RDL DATE 12-1-14

L15672000 + 02001

Calculate Interior Stringer LLDF - per AASHTO Table 3.23.1

Timber Plank Floor

<u>Stringer</u>	<u>S (ft)</u>	<u>LLDF = S/4.0 (1 lane)</u>
1 st interior	2.375	0.5938 ✓
Other interior	2	0.5000 ✓

BRIDGE POSTED
FOR "ONE CAR
AT A TIME ON
BRIDGE."Calculate Weight of Guardrail

W-rail : - Per MDT Detail 606-88, say 20" wide sheet metal makes 12" W-shape
- Assume 10ga sheet, $t = 0.138$ in

$$w = (490 \text{ pcf}) \left(\frac{0.138}{12} \text{ ft} \right) \left(\frac{20}{12} \text{ ft} \right) = 9.4 \text{ plf} \checkmark$$

Posts : HSS 3x3x 3/16 (thickness assumed) @ 8' o.c.

$$w = 6.87 \text{ plf} \times 3.417 \text{ ft} / 8 \text{ ft} = 2.9 \text{ plf} \checkmark$$

L Rail : L3x3x 1/4

$$w = 4.90 \text{ plf} \checkmark$$

Handrail : 1 3/4" ϕ Pipe (assume HSS 1.660 x 0.140)

$$w = 2.3 \text{ plf} \checkmark$$

(CONSERVATIVE
ONLY ON ONE
SIDE)

$$\text{Total Railing Weight} = 9.4 + 2.9 + 4.9 + 2.3 = \underline{\underline{19.5 \text{ plf}}} \checkmark$$

PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 12/31/13CHECKED BY RDL DATE 12-2-14

L 15672000 + 02001

Calculate "Equivalent Deck Thickness"

Virtis does not allow wood decks on trusses, but does allow concrete decks.

∴ Calculate a concrete thickness that will provide the same DL as the wood deck.

$$4'' \text{ thick deck planks: } p = (50 \text{ pcf}) \left(\frac{4}{12} \text{ ft} \right) = 16.667 \text{ psf}$$

$$\text{Concrete: } p = (150 \text{ pcf})(t) = 16.667 \text{ psf}$$

$$t = 0.111 \text{ ft} = 1.333 \text{ in} \quad \checkmark$$

Calculate Weight of Running Planks

2½" thick running planks full width of bridge

$$\gamma = 50 \text{ pcf} \rightsquigarrow \frac{(2\frac{1}{2})}{12} = 0.2083 \rightsquigarrow \left(\frac{0.2083}{\text{ft}} \right) \left(\frac{50}{\text{lb/ft}^3} \right) = 10.42 \text{ PSF}$$

⇒ Apply as wearing surface load in Virtis

PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 12/31/13CHECKED BY RDL DATE 12-2-14

L15672000+02001

Utility Pipe (Water/Sewer) Weight

Pipe circumference = 27.5 in ✓

$$C = \pi d \Rightarrow d = \frac{27.5 \text{ in}}{\pi} = 8.75 \text{ in Outside diameter}$$

Pipe 8 Std. ✓ O.D. = 8.63 in , I.D. = 7.98 in
w = 28.6 plf

$$\text{Water area} = \pi \left(\frac{7.98 \text{ in}}{2} \right)^2 = 50.0 \text{ in}^2$$

$$\text{Max water weight} = 62.4 \text{ pcf} \times \frac{50}{144} \text{ ft}^2 = 21.7 \text{ plf} \checkmark$$

$$\text{Pipe Empty Weight} = \underline{28.6 \text{ plf}} \checkmark$$

$$\text{Pipe Full Weight} = 28.6 \text{ plf} + 21.7 \text{ plf} = \underline{50.3 \text{ plf}} \checkmark$$

* Pipe is hung on Stringer 2 at Floorbeams.

$$\text{Stringer point load} = 50.3 \text{ plf} \times 17 \text{ ft} = \underline{\underline{855 \text{ lbs}}} \checkmark$$

Wood Conduit Weight

"2x" - 20" wood "box" Supported by Floorbeams @ Upstream end of floorbeam

$$A = \frac{1}{2} \text{ in} \times 20 \text{ in} \times 4 \text{ sides} = 120 \text{ in}^2$$

$$W = (50 \text{ pcf}) \left(\frac{120}{144} \text{ ft}^2 \right) = 41.7 \text{ plf} \checkmark$$

$$\text{Floorbeam point load} = 41.7 \text{ plf} \times 17 \text{ ft} = \underline{\underline{709 \text{ lbs}}} \checkmark$$



Wood box conduit on
end of Floorbeams

Water/Sewer pipe hung
from Stringer & near
Floorbeams

PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 12/30/13CHECKED BY RDL DATE 12-2-14

L15672000+02001

Calculate Weight of Walkway RailingRails: 5 ~ 1 3/4" ϕ Pipes

$$w = 5 \times 2.3 \text{ plf} = 11.5 \text{ plf} \quad \checkmark$$

L Posts: L 3x3x3/16 x 4'-4 3/4" @ 13' o.c.

$$w = 3.71 \text{ plf} \times 4.40 \text{ ft} / 13 \text{ ft} = 1.3 \text{ plf} \quad \checkmark$$

Pipe Hangers: 1 1/4" ϕ Pipe x 7'-6" @ 13' o.c.

$$w = 1.7 \text{ plf} \times 7.5 \text{ ft} / 13 \text{ ft} = 1.0 \text{ plf} \quad \checkmark$$

Channel hanger: C5x6.7 x 5'-3" long @ 13' o.c.

$$w = 6.7 \text{ plf} \times 5.25 \text{ ft} / 13 \text{ ft} = 2.7 \text{ plf} \quad \checkmark$$

Double channel hanger support: Assume JI 5x6.7

$$w = 2 \times 6.7 = 13.4 \text{ plf} \quad (\text{welded directly to main truss}) \quad \checkmark$$

$$\text{Total Walkway Rail Weight} = 11.5 + 1.3 = 12.8 \text{ plf} \quad \checkmark$$

$$\text{Total Hanger Weight} = 1.0 + 2.7 + 13.4 = 17.1 \text{ plf} \quad \checkmark$$

$$\text{Wood Running Boards: } w = 50 \text{ pcf} \times \frac{2.75}{12} \text{ ft} = 11.46 \text{ psf} \quad \checkmark \quad (2 \frac{3}{4}'' \times 11 \frac{3}{4}'' \text{ boards})$$

Walkway Beams: S 4x7.7 w/ 3/16" x 6" top ϕ

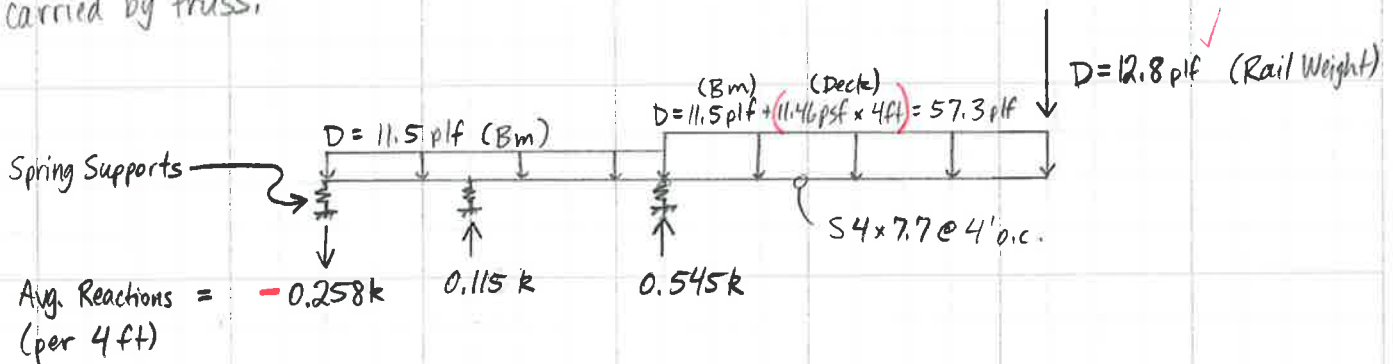
$$w = 7.7 \text{ plf} + (490 \text{ pcf}) \left(\frac{3/16}{12} \text{ ft} \right) \left(\frac{6}{12} \text{ ft} \right) = 11.5 \text{ plf} \quad \checkmark$$



L15672000 + 02001

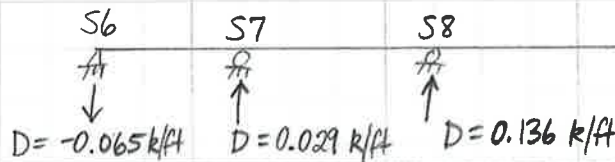
Sidewalk Dead Load

* Hanger assembly weight not included. Assume hanger weight carried by truss.



Reactions determined in Visual Analysis (attached). **OK ✓**

DL Applied to Stringers :



Stringer 6: $D = \frac{-0.258k}{4ft} = -0.065 k/ft$ ✓

Stringer 7: $D = \frac{0.115k}{4ft} = 0.029 k/ft$ ✓

Stringer 8: $D = \frac{0.545k}{4ft} = 0.136 k/ft$ ✓

(Stringer S6 is assumed to have a connection to the walkway beams that is capable of supporting an uplift reaction.) ✓

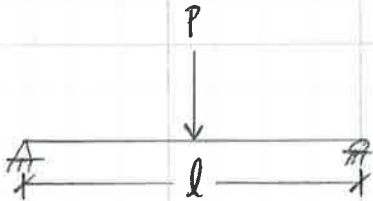
PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 1/3/14CHECKED BY RDL DATE 12-2-14

L15672000 + 02001

Stringer "Spring Support" Stiffnesses



$$\Delta = \frac{Pl^3}{48EI}$$

✓ (@ CENTER BUT STIFFNESSES WILL REMAIN PROPORTIONAL ALONG LENGTHS OF BEAMS)

$$k = \frac{P}{\Delta} = \frac{48EI}{l^3}$$

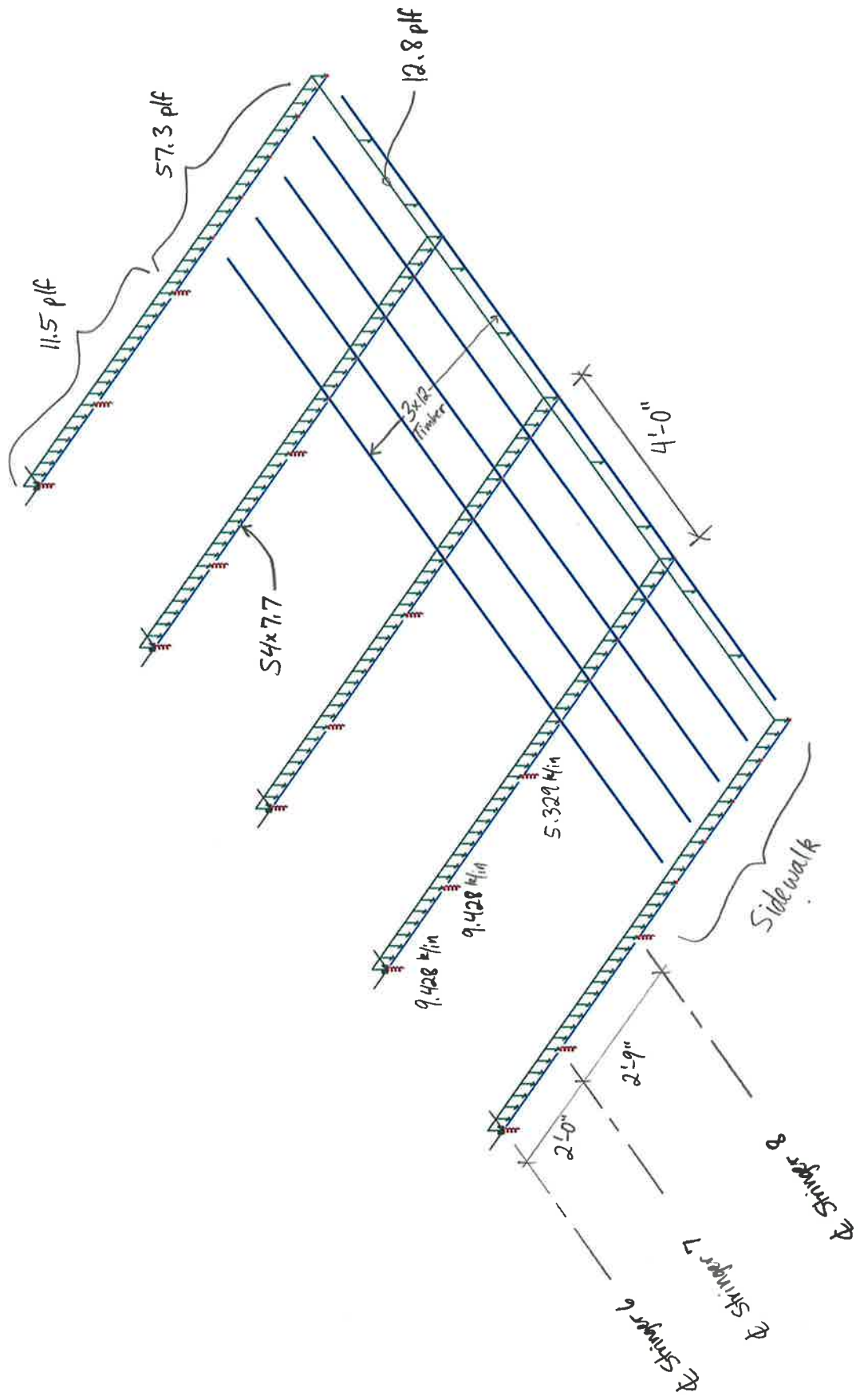
- Exterior Stringer Support: C 8x11.5 $I = 32.5 \text{ in}^4$

$$k = \frac{48(29,000 \text{ ksi})(32.5 \text{ in}^4)}{(17 \times 12 \text{ in})^3} = 5.329 \text{ k/in} \checkmark$$

- Interior Stringer Support: S 8x18.4 $I = 57.5 \text{ in}^4$

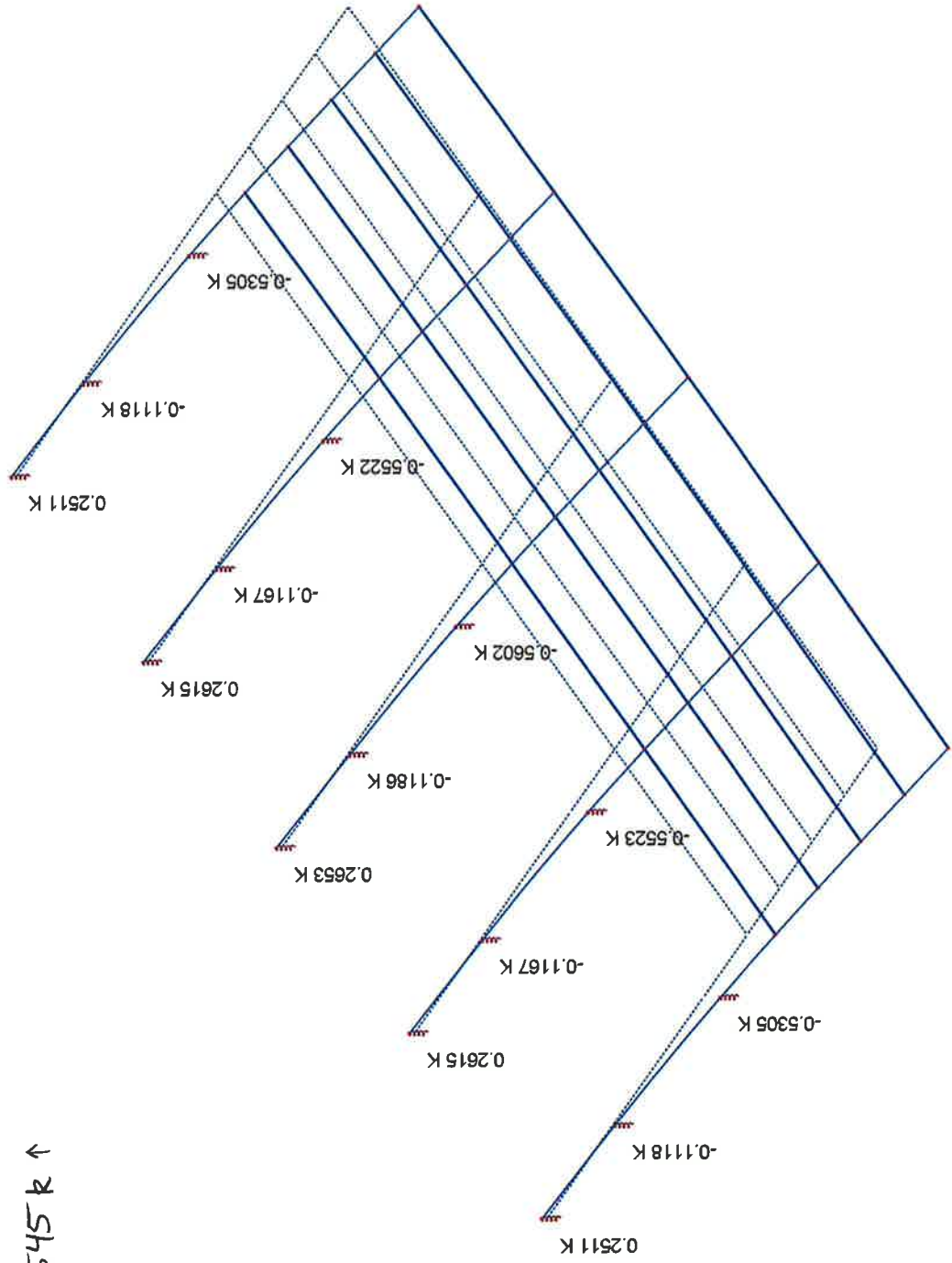
$$k = \frac{48(29,000 \text{ ksi})(57.5 \text{ in}^4)}{(17 \times 12 \text{ in})^3} = 9.428 \text{ k/in} \checkmark$$

Visual Analysis model of sidewalk



Average Reactions (per 4 ft)

- S6 0.258 k ↓
- S7 0.115 k ↑
- S8 0.545 k ↑





L15672000 +02001

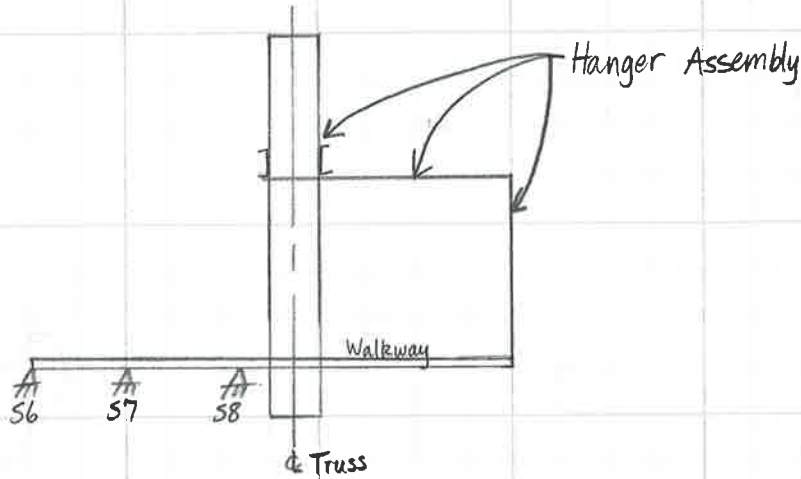
PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 1/3/14

CHECKED BY RDL DATE 12-2-14

Hanger Assembly Weight - carried directly by truss



Hanger Assembly Weight = 17.1 plf ✓

Load at Panel Points = 17.1 plf × 17 ft = 291 lbs ✓

∴ Apply 291 lbs at panel points on Right (Upstream) Truss ✓
(L1-L6)

and 145 lbs at L0 ∴ L7



Calculate Weight of Portal Framing

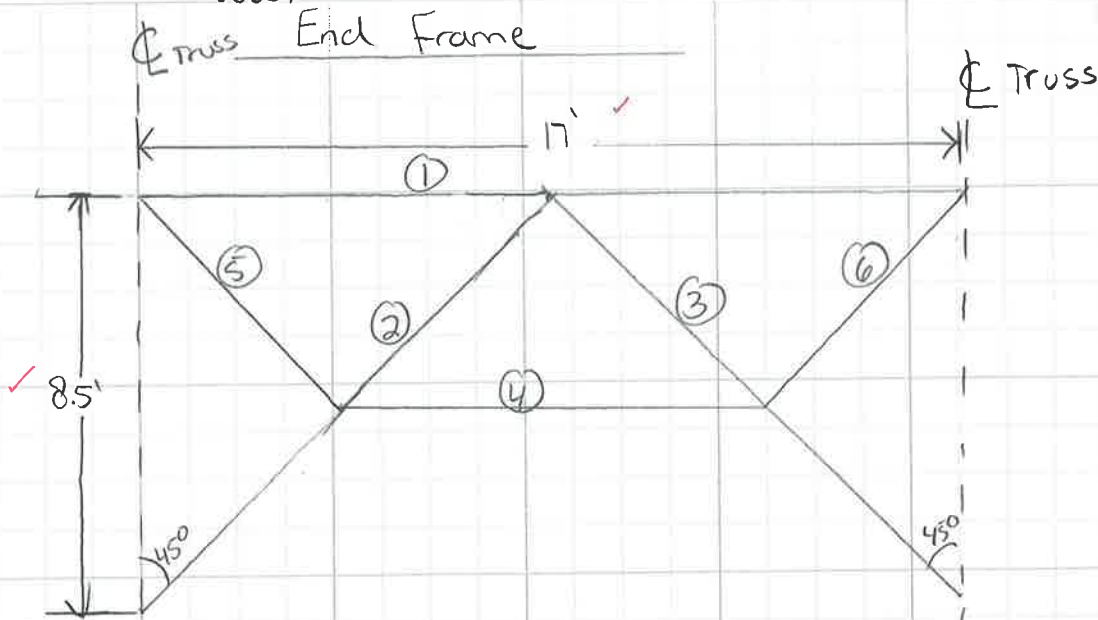
PROJECT MDT Bridge Load Rating 13-035

SHEET NO. _____ OF _____

CALCULATED BY JLW DATE 11/24/14

CHECKED BY BEL DATE 11/25/14

#15 Swan River



Assumptions ✓ (based on photos)

- All members are $L 2 \times 2 \times 1/4$ ✓
- 45° Angle between Truss members 2 & 3 ✓
- members 2 & 3 connect @ midpoint of member 1 ✓
- members 3, 4, & 5 connect @ midpoint of members 2 & 3 ✓

$L 2 \times 2 \times 1/4$ weight = 3.19 plf ✓

Length of members 2 & 3: $\sqrt{8.5^2 + 8.5^2} = 12.02 \text{ ft}$ ✓

location of members 2, 4, & 5 connection: $\sin(45^\circ) = \frac{x}{6.01 \text{ ft}} \leftarrow \frac{12.02'}{2}$

$x = 4.25 \text{ ft}$ ✓

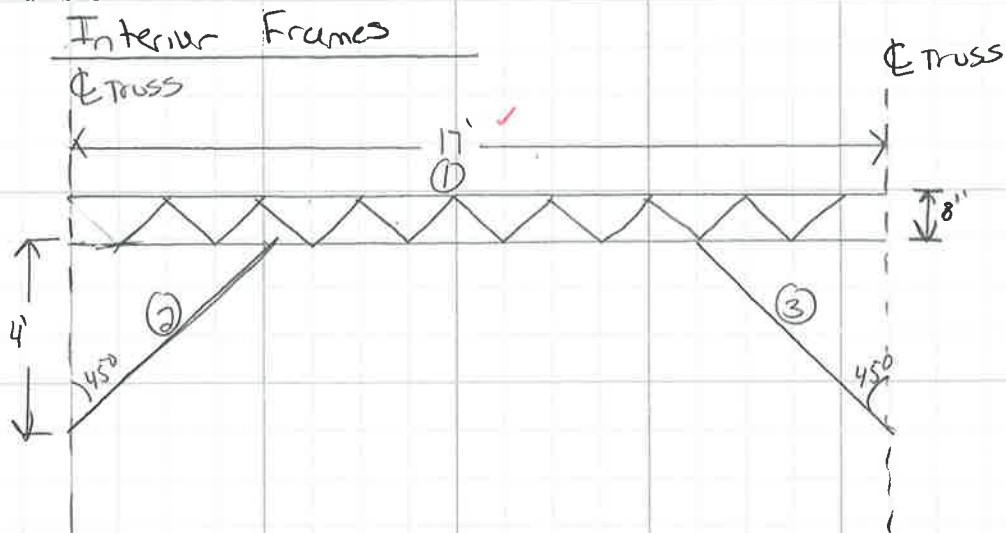
length of (1) = $4.25 \text{ ft} (2) = 8.5 \text{ ft}$ ✓

length of 5 & 6 = $\sqrt{4.25^2 + 4.25^2} = 6.01 \text{ ft}$ ✓

load to panel point 1 = $2(3.19 \text{ plf}) \left(\frac{17'}{2} + 12.02' + 6.01' + \frac{8.5'}{2} \right)$
 $(11) = 6.38 \text{ plf} (30.78 \text{ ft}) = 196.38 \text{ lbs}$ ✓



#15 Swan River



Assumptions: (based on photos)

- members 1, 2, + 3 are $\text{JL } 2 \times 2 \times \frac{1}{4}$ ✓
- 45° angle between Truss + members 2 + 3 ✓
- lattice bracing $1\frac{1}{2}$ " wide + $\frac{1}{4}$ " thick, + 10" long ✓
- Scaling from picture, members 2 + 3 connect to member 1
 ≈ 4 ft away from Truss

length of members 2 + 3 : $\sqrt{4^2 + 4^2} = 5.66 \text{ ft}$ ✓

equivalent uniform weight of lattice bracing: $\frac{1.5 \text{ m} (0.25 \text{ m}) (10 \text{ m}) (490 \text{ pcf}) (34) \text{ braces} (1 \text{ ft} / 12 \text{ in})^3}{17 \text{ ft}} = \frac{36.15 \text{ lbs}}{17 \text{ ft}}$

$= 2.13 \text{ pcf}$ ✓

load to panel point: $(4(3.19 \text{ pcf}) + 2.13 \text{ pcf}) (17 \text{ ft} / 2) + 2(3.19 \text{ pcf})(5.66 \text{ ft})$

$= 14.89 \text{ pcf} (8.5 \text{ ft}) + 6.38 \text{ pcf} (5.66 \text{ ft})$

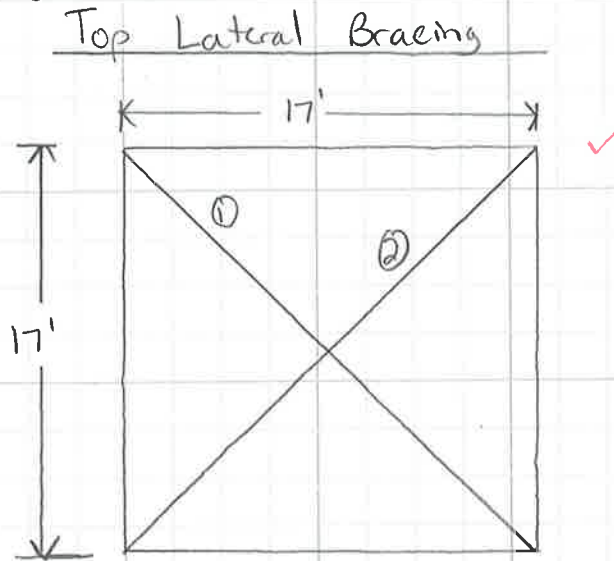
$= 162.63 \text{ lbs}$ ✓

PROJECT MDT Bridge Load Rating 13-035

SHEET NO. _____ OF _____

CALCULATED BY JLW DATE 11/24/14CHECKED BY BEL DATE 11/25/14

#15 Swan River

Assumptions

- members 1+2 are a circular 1" ϕ rod ✓

$$\text{Diagonal length} = \sqrt{17^2 + 17^2} = 24.04 \text{ ft} \checkmark$$

$$\text{Area of rod} = \frac{\pi d^2}{4} = \frac{\pi (1 \text{ in})^2}{4} = 0.785 \text{ in}^2 \left(\frac{1 \text{ ft}}{12 \text{ in}} \right)^2 = 0.005 \text{ ft}^2$$

$$\text{load to panel points } 1+6 = 0.005 \text{ ft}^2 \left(\frac{24.04 \text{ ft}}{2} \right) (490 \text{ pcf}) = 32.13 \text{ lbs} \checkmark$$

$$\text{load to panel points } 2-5 = 0.005 \text{ ft}^2 \left(2 \left(\frac{24.04 \text{ ft}}{2} \right) \right) (490 \text{ pcf}) = 64.25 \text{ lbs} \checkmark$$

Total Load to Panel Points from Portal Framing

$$\text{Panel Points } (11, 16) \checkmark \quad 1+6: 196.38 \text{ lbs} + 32.13 \text{ lbs} = \boxed{228.51 \text{ lbs}} \checkmark$$

$$\text{Panel Points } (12-15) \checkmark \quad 2-5: 162.63 \text{ lbs} + 64.25 \text{ lbs} = \boxed{226.88 \text{ lbs}} \checkmark$$



ONE CAR
AT A TIME
ON BRIDGE



PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 11/25/14

CHECKED BY RDL DATE 12-2-14

Summary - Total Panel Point Loads to be Applied to Truss in BrR

Panel Point	Sidewalk Hanger Assembly*	Portal Framing
L0	0.145 k ✓	-
L1	0.291 k	-
L2	0.291 k	-
L3	0.291 k	-
L4	0.291 k	-
L5	0.291 k	-
L6	0.291 k	-
L7	0.145 k ✓	-
U1	-	0.229 k
U2	-	0.227 k
U3	-	0.227 k
U4	-	0.227 k
U5	-	0.227 k
U6	-	0.229 k

* SE (Upstream) truss only



L15672000+02001

Member Cross Sectional Properties

PROJECT MDT Load Rating SEI 13-035

SHEET NO. _____ OF _____

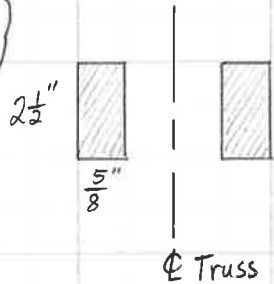
CALCULATED BY BBL DATE 12/31/13

CHECKED BY RDL DATE 12-2-14

BC1 & BC2

2 ~ 2 1/2" x 5/8" bars

BC1 has 2/32" of section loss on the inside rail per MDT Assessment (2012).



$$A_g = A_n = 2 \times 2\frac{1}{2} \text{ in} \times \frac{5}{8} \text{ in} = 3.125 \text{ in}^2 \checkmark$$

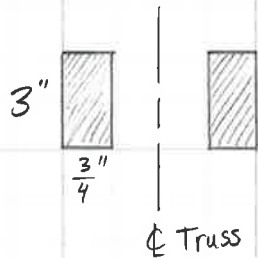
$$I_x = 2 \times \frac{(5/8 \text{ in})(2\frac{1}{2} \text{ in})^3}{12} = 1.628 \text{ in}^4 \checkmark$$

$$I_y = 2 \times \frac{(2\frac{1}{2} \text{ in})(\frac{5}{8} \text{ in})^3}{12} = 0.102 \text{ in}^4 \checkmark$$

$$* \text{BC1 } A_n = 2\frac{1}{2} \times \frac{5}{8} + (2\frac{1}{2} - \frac{1}{16})(\frac{5}{8} - \frac{1}{16}) = 2.934 \text{ in}^2 \checkmark$$

BC3

2 ~ 3" x 3/4" bars



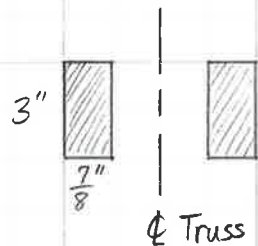
$$A_g = A_n = 2 \times 3 \text{ in} \times \frac{3}{4} \text{ in} = 4.5 \text{ in}^2 \checkmark$$

$$I_x = 2 \times \frac{(3/4 \text{ in})(3 \text{ in})^3}{12} = 3.375 \text{ in}^4$$

$$I_y = 2 \times \frac{(3 \text{ in})(3/4 \text{ in})^3}{12} = 0.211 \text{ in}^4 \checkmark$$

BC4

2 ~ 3" x 7/8" bars



$$A_g = A_n = 2 \times 3 \text{ in} \times \frac{7}{8} \text{ in} = 5.25 \text{ in}^2 \checkmark$$

$$I_x = 2 \times \frac{(7/8 \text{ in})(3 \text{ in})^3}{12} = 3.938 \text{ in}^4 \checkmark$$

$$I_y = 2 \times \frac{(3 \text{ in})(7/8 \text{ in})^3}{12} = 0.335 \text{ in}^4$$



L15672000+02001

PROJECT MDT Bridge Rating SEI 13-035

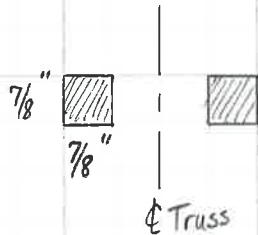
SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 12/31/13

CHECKED BY RDL DATE 12-2-14

VW1

2 ~ $\frac{7}{8}$ " x $\frac{7}{8}$ " bars

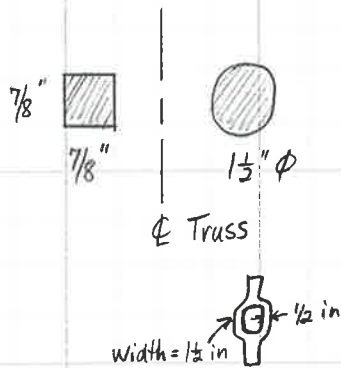
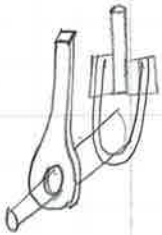


$$A_g = A_n = 2 \times \left(\frac{7}{8} \text{ in}\right) \left(\frac{7}{8} \text{ in}\right) = 1.531 \text{ in}^2 \checkmark$$

$$I_x = 2 \times \frac{\left(\frac{7}{8} \text{ in}\right) \left(\frac{7}{8} \text{ in}\right)^3}{12} = 0.098 \text{ in}^4 \checkmark$$

$$I_y = 0.098 \text{ in}^4 \checkmark$$

VW6 R (upstream truss) - Retrofitted member



$$A_g = \frac{7}{8} \text{ in} \times \frac{7}{8} \text{ in} + \pi \left(\frac{1.5 \text{ in}}{2}\right)^2 = 2.533 \text{ in}^2 \checkmark$$

$$I_x = \frac{\left(\frac{7}{8} \text{ in}\right) \left(\frac{7}{8} \text{ in}\right)^3}{12} + \pi \frac{\left(\frac{1.5 \text{ in}}{2}\right)^4}{64} = \cancel{0.064} \text{ in}^4 + 0.297 \text{ in}^4$$

$$I_y = \cancel{0.064} \text{ in}^4 + 0.297 \text{ in}^4$$

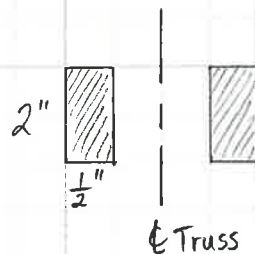
$$U \text{ bolt area} = 2 \times 0.44 \text{ in}^2 = 0.88 \text{ in}^2 < A_{rod} = 1.767 \text{ in}^2$$

$$\text{Turnbuckle area} = \frac{1}{2} \text{ in} \times \frac{1}{2} \text{ in} \times 2 = 1.5 \text{ in}^2 < A_{rod} = 1.767 \text{ in}^2$$

$$A_n = 0.88 \text{ in}^2 + \frac{7}{8} \text{ in} \times \frac{7}{8} \text{ in} = 1.646 \text{ in}^2 \checkmark$$

DW1 - Same member configuration as BC2

DW2 2 ~ 2" x $\frac{1}{2}$ " bars



$$A_g = A_n = 2 \times 2 \text{ in} \times \frac{1}{2} \text{ in} = 2 \text{ in}^2 \checkmark$$

$$I_x = 2 \times \frac{\left(\frac{1}{2} \text{ in}\right) \left(2 \text{ in}\right)^3}{12} = 0.667 \text{ in}^4 \checkmark$$

$$I_y = 2 \times \frac{\left(2 \text{ in}\right) \left(\frac{1}{2} \text{ in}\right)^3}{12} = 0.042 \text{ in}^4 \checkmark$$



L 15672000+02001

PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

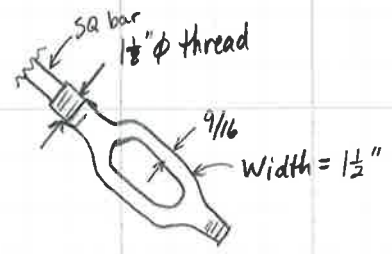
CALCULATED BY BBL DATE 12/31/13

CHECKED BY RDL DATE 12-2-14

DW3 & DW4

- Same member configuration as VW1 ✓

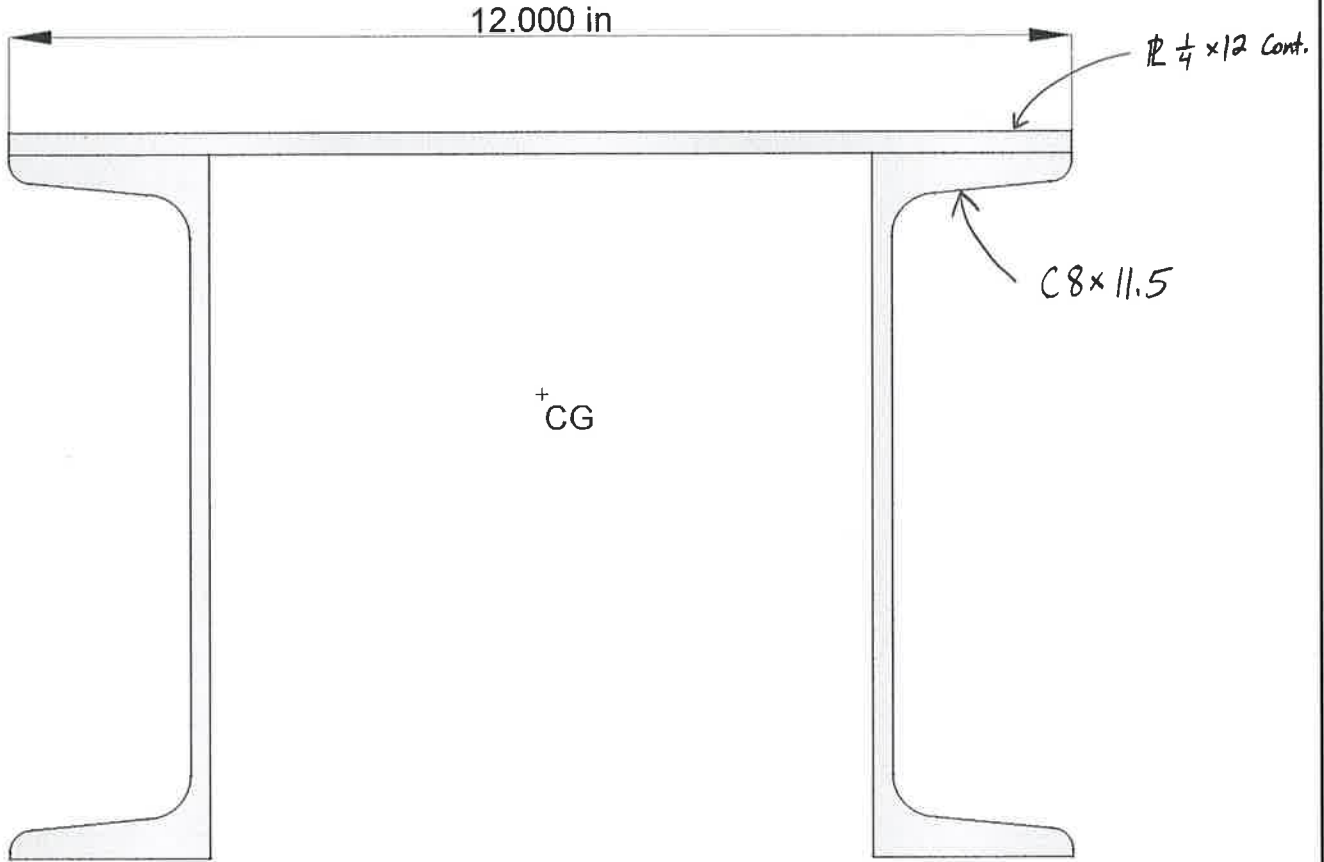
Turnbuckle Area:



$$A_{\text{thread}} = \pi \left(\frac{1\frac{1}{8} \text{ in}}{2} \right)^2 = 0.994 \text{ in}^2 > A_{\text{bar}} = \frac{1.53 \text{ in}^2}{2} = 0.77 \text{ in}^2$$

$$A_{\text{turnbuckle}} = \frac{9}{16} \text{ in} \times \frac{1}{2} \text{ in} \times 2 = 1.688 \text{ in}^2 > A_{\text{bar}}$$

OK
✓



Overall Dimensions

Width	12.000 in
Height	8.250 in
Perimeter	71.886 in

Geometric Properties

Iy	168.784 in ⁴
Ix	102.940 in ⁴
J	0.192 in ⁴
ry	4.125 in
rx	3.221 in
A	9.921 in ²
Sy Right	28.131 in ³
Sy Left	28.131 in ³
Sx Bottom	19.617 in ³
Sx Top	34.283 in ³
Centroid Y	0.889 in
Centroid X	2.167 in

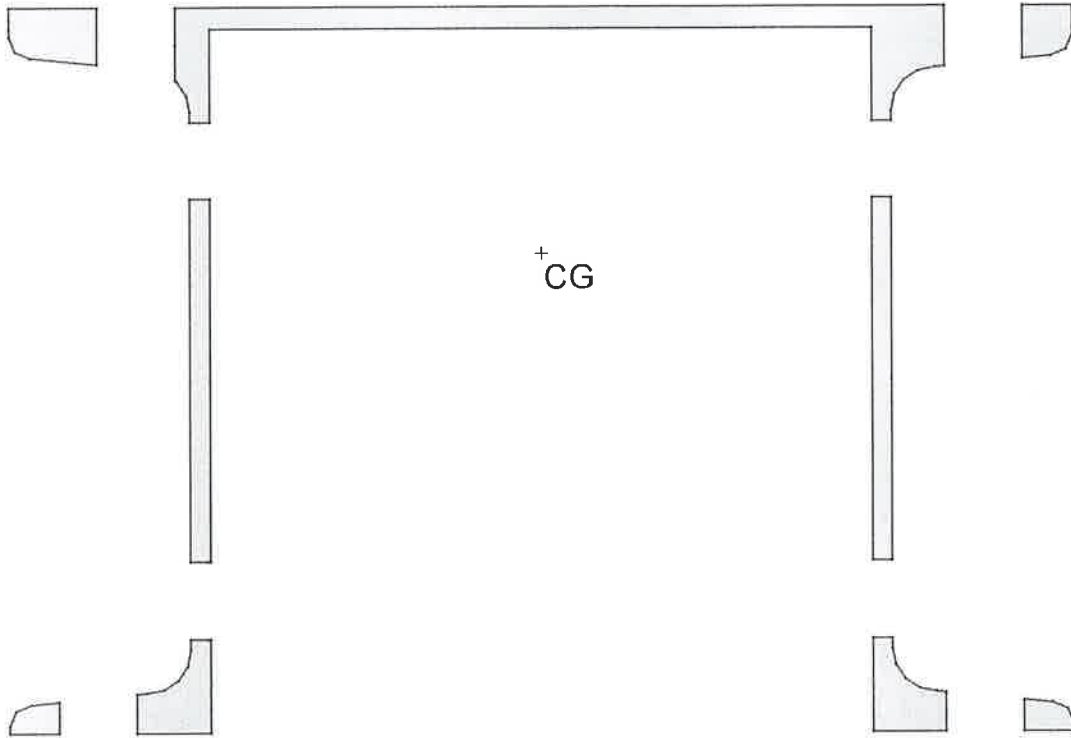
Material Properties

E	29000.000 Ksi
---	---------------

Plastic Properties

Fy Base	36.000 Ksi
Zy	42.488 in ³
Zx	27.781 in ³
PNA-Y	3.331 in

PNA-X	2.167 in
Polar Properties	
Ip	271.724 in ⁴
rp	5.233 in
Principal Properties	
I2	102.940 in ⁴
I1	168.784 in ⁴
Ixy	-0.000 in ⁴
Theta	90.000 deg



Overall Dimensions

Width	12.000 in
Height	8.250 in
Perimeter	54.944 in

Geometric Properties

Iy	112.926 in ⁴
Ix	70.338 in ⁴
ry	3.943 in
rx	3.112 in
A	7.265 in ²
Sy Right	18.806 in ³
Sy Left	18.836 in ³
Sx Bottom	12.950 in ³
Sx Top	24.956 in ³
Centroid Y	1.073 in
Centroid X	2.162 in

Material Properties

E	29000.000 Ksi
---	---------------

Plastic Properties

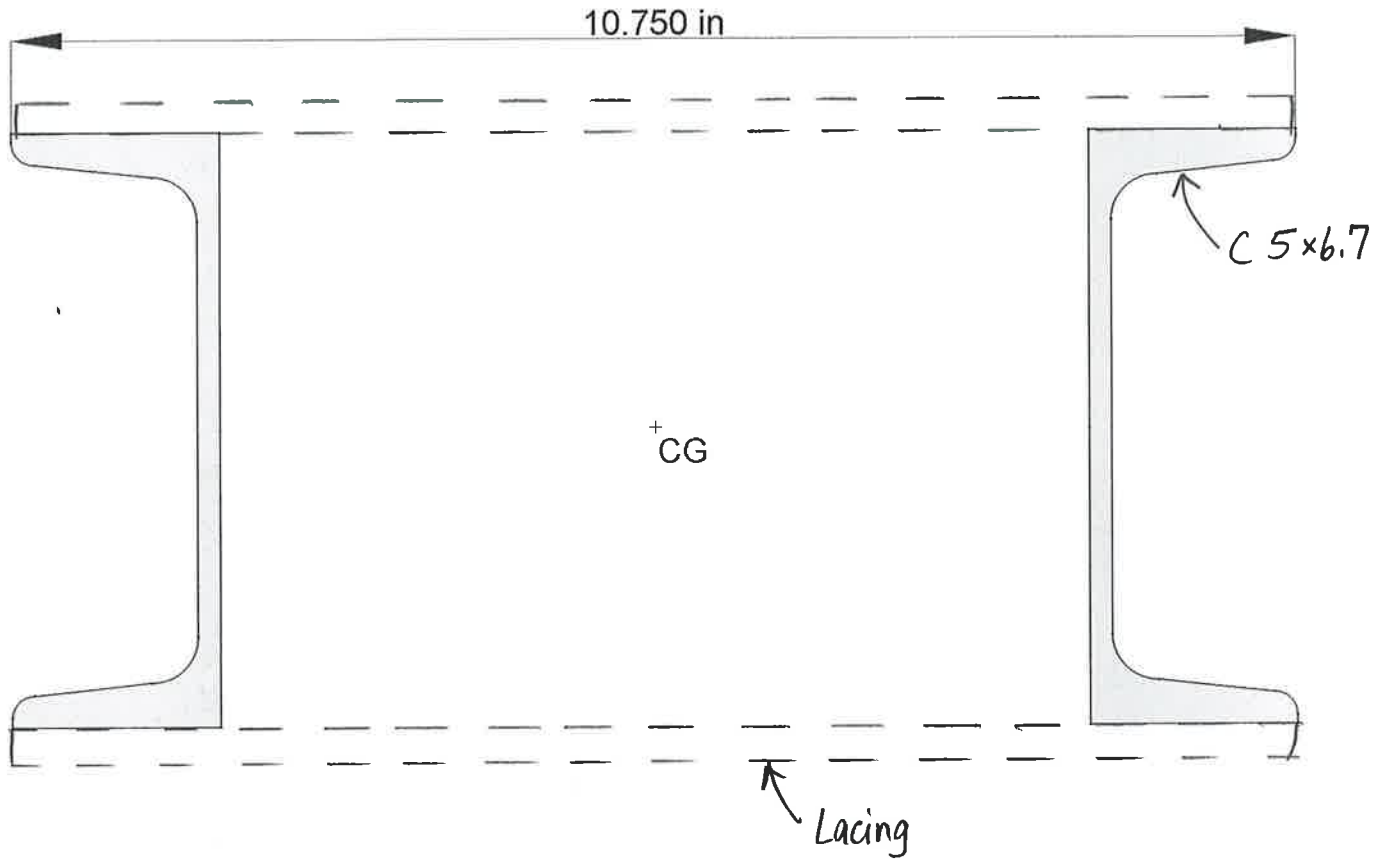
Fy Base	36.000 Ksi
Zy	32.087 in ³
Zx	19.419 in ³
PNA-Y	3.418 in
PNA-X	2.262 in

Polar Properties

Ip	183.264 in ⁴
rp	5.023 in

Principal Properties

I2	70.337 in ⁴
I1	112.927 in ⁴
Ixy	-0.163 in ⁴
Theta	89.780 deg



Overall Dimensions

Width	10.750 in
Height	5.000 in
Perimeter	31.807 in

Geometric Properties

Iy	69.589 in ⁴
Ix	15.426 in ⁴
J	0.055 in ⁴
ry	4.155 in
rx	1.956 in
A	4.032 in ²
Sy Right	12.947 in ³
Sy Left	12.947 in ³
Sx Bottom	6.170 in ³
Sx Top	6.170 in ³
Centroid Y	-0.036 in
Centroid X	2.642 in

Material Properties

E	29000.000 Ksi
---	---------------

Plastic Properties

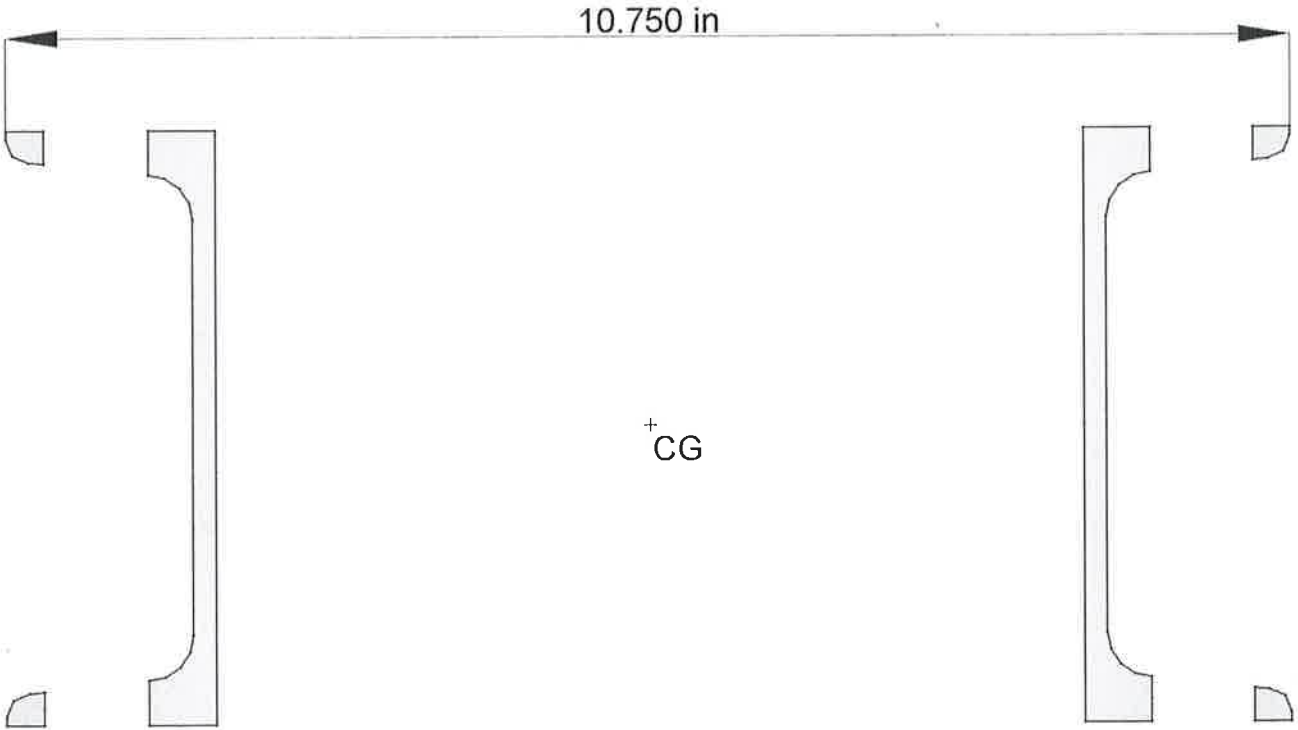
Zy	16.627 in ³
Zx	7.312 in ³
PNA-Y	-0.036 in
PNA-X	2.642 in

Polar Properties

Ip	85.015 in ⁴
rp	4.592 in

Principal Properties

I2	15.426 in ⁴
I1	69.589 in ⁴
Ixy	0.000 in ⁴
Theta	90.000 deg



Overall Dimensions

Width	10.750 in
Height	5.000 in
Perimeter	27.406 in

Geometric Properties

Iy	45.251 in ⁴
Ix	9.172 in ⁴
ry	3.959 in
rx	1.782 in
A	2.887 in ²
Sy Right	8.419 in ³
Sy Left	8.419 in ³
Sx Bottom	3.669 in ³
Sx Top	3.669 in ³
Centroid Y	-0.036 in
Centroid X	2.642 in

Material Properties

E	29000.000 Ksi
---	---------------

Plastic Properties

Zy	11.356 in ³
Zx	4.639 in ³
PNA-Y	-0.036 in
PNA-X	2.642 in

Polar Properties

Ip	54.423 in ⁴
rp	4.342 in
Principal Properties	
I2	9.172 in ⁴
I1	45.251 in ⁴
Ixy	0.000 in ⁴
Theta	90.000 deg



PROJECT MDT Bridge Rating SEI 13-035

SHEET NO. _____ OF _____

CALCULATED BY BBL DATE 1/3/14

CHECKED BY RDL DATE 12-2-14

L15672000 + 02001

Reduced Capacity Members

VW3L - Vehicular damage caused loss of straightness, but no section loss.
 Reduce compression capacity to 60% of full capacity. (PER ENGINEERING JUDGEMENT)

DW1, DW2, DW3, DW4, DWS, DW6, VW1, VW6

- MDT Fracture Critical Inspection (2010) states that counter members are "loose," and some eyes have hairline forge cracks.
 Reduce tension capacity by 10%. (PER ENGINEERING JUDGEMENT)

Member	Full Capacity (k) (As reported by Virtis)	Reduction (%)	Final Capacity (k) *
VW3L	(COMPR.) -68.58 k	40% ✓	-41.15 k
DW1, DW6	(TENS.) 93.75 k	10%	84.38 k
DW2, DWS	60.00 k	10% ✓	54.00 k
DW3, DW4	45.93 k	10%	41.34 k ✓
VW1, VW6	45.93 k	10%	41.34 k
VW6 R	49.38 k	10%	44.44 k

* Input these capacities into "MemberOfInterest" command's "OverrideCapacity" subcommand. ✓

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30 , A2 = 2.17
Operating:
 A1 = 1.30 , A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

NW Truss (Downstream)
Member Full Capacities

Member	Truss Element	DL Force (kip)	LL Force				Capacity		Adj Veh Demand		One Lane LLDF	Multi Lane LLDF	Inv RF	Opr RF	Legal Opr RF	Permit Inv RF	Permit Opr RF
			Comp. Inv / Opr / Legal Permit Inv / Permit Opr (kip)	IF	Tens. Inv / Opr / Legal Permit Inv / Permit Opr (kip)	IF	Comp. (kip)	Tens. (kip)	Comp. (kip)	Tens. (kip)							
BC1	Lower-Chord	20.61	////		45.39 / 45.39	1.20		88.02		0.665		0.775	1.294				
		20.61	////		45.39 / 45.39	1.20		88.02		0.665		0.775	1.294				
		20.61	////		45.39 / 45.39			88.02		0.665		0.934	1.559				
		20.61	////		45.39 / 45.39			88.02		0.665		0.934	1.559				
BC2	Lower-Chord	20.61	////		45.39 / 45.39	1.20		93.75		0.665		0.848	1.415				
		20.61	////		45.39 / 45.39	1.20		93.75		0.665		0.848	1.415				
		20.61	////		45.39 / 45.39			93.75		0.665		1.021	1.705				
		20.61	////		45.39 / 45.39			93.75		0.665		1.021	1.705				
BC3	Lower-Chord	34.46	////		74.12 / 74.12	1.20		135.00		0.665		0.699	1.168				
		34.46	////		74.12 / 74.12	1.20		135.00		0.665		0.699	1.168				
		34.46	////		74.12 / 74.12			135.00		0.665		0.842	1.407				
		34.46	////		74.12 / 74.12			135.00		0.665		0.842	1.407				
BC4	Lower-Chord	41.00	////		84.15 / 84.15	1.20		157.50		0.665		0.711	1.188				
		41.00	////		84.15 / 84.15	1.20		157.50		0.665		0.711	1.188				
		41.00	////		84.15 / 84.15			157.50		0.665		0.857	1.432				
		41.00	////		84.15 / 84.15			157.50		0.665		0.857	1.432				
BC5	Lower-Chord	34.46	////		74.12 / 74.12	1.20		135.00		0.665		0.699	1.168				
		34.46	////		74.12 / 74.12	1.20		135.00		0.665		0.699	1.168				
		34.46	////		74.12 / 74.12			135.00		0.665		0.842	1.407				
		34.46	////		74.12 / 74.12			135.00		0.665		0.842	1.407				
BC6	Lower-Chord	20.61	////		45.39 / 45.39	1.20		93.75		0.665		0.848	1.415				
		20.61	////		45.39 / 45.39	1.20		93.75		0.665		0.848	1.415				
		20.61	////		45.39 / 45.39			93.75		0.665		1.021	1.705				
		20.61	////		45.39 / 45.39			93.75		0.665		1.021	1.705				
BC7	Lower-Chord	20.61	////		45.39 / 45.39	1.20		88.02		0.665		0.775	1.294				
		20.61	////		45.39 / 45.39	1.20		88.02		0.665		0.775	1.294				
		20.61	////		45.39 / 45.39			88.02		0.665		0.934	1.559				
		20.61	////		45.39 / 45.39			88.02		0.665		0.934	1.559				
TC2	Upper-Chord	-34.46	-74.12 / -74.12	1.20	////			-232.63		0.665		1.456	2.431				
		-34.46	-74.12 / -74.12	1.20	////			-232.63		0.665		1.456	2.431				
		-34.46	-74.12 / -74.12		////			-232.63		0.665		1.754	2.929				
		-34.46	-74.12 / -74.12		////			-232.63		0.665		1.754	2.929				
TC3	Upper-Chord	-41.45	-87.73 / -87.73	1.20	////			-232.63		0.665		1.171	1.955				
		-41.45	-87.73 / -87.73	1.20	////			-232.63		0.665		1.171	1.955				
		-41.45	-87.73 / -87.73		////			-232.63		0.665		1.410	2.355				
		-41.45	-87.73 / -87.73		////			-232.63		0.665		1.410	2.355				
TC4	Upper-Chord	-41.91	-87.41 / -87.20	1.20	////			-232.63		0.665		1.171	1.960				
		-41.91	-87.41 / -87.20	1.20	////			-232.63		0.665		1.171	1.960				
		-41.91	-87.41 / -87.20		////			-232.63		0.665		1.411	2.362				
		-41.91	-87.41 / -87.20		////			-232.63		0.665		1.411	2.362				
TC5	Upper-Chord	-41.45	-87.73 / -87.73	1.20	////			-232.63		0.665		1.171	1.955				
		-41.45	-87.73 / -87.73	1.20	////			-232.63		0.665		1.171	1.955				
		-41.45	-87.73 / -87.73		////			-232.63		0.665		1.410	2.355				
		-41.45	-87.73 / -87.73		////			-232.63		0.665		1.410	2.355				
TC6	Upper-Chord	-34.46	-74.12 / -74.12	1.20	////			-232.63		0.665		1.456	2.431				
		-34.46	-74.12 / -74.12	1.20	////			-232.63		0.665		1.456	2.431				
		-34.46	-74.12 / -74.12		////			-232.63		0.665		1.754	2.929				
		-34.46	-74.12 / -74.12		////			-232.63		0.665		1.754	2.929				
VW1	Vertical	7.18	////		39.06 / 39.06	1.30		45.93		0.665		0.499	0.833				

NW Truss (Downstream)
Member Full Capacities (cont.)

LL Scale Factor = 1.00
Adjacent Vehicle LL Factor = 0.00
Inventory:
A1 = 1.30, A2 = 2.17
Operating:
A1 = 1.30, A2 = 1.30
Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.18		///		39.06 / 39.06 ///	1.30		45.93			0.665	0.499	0.833		
		7.18		///		39.06 / 39.06 ///			45.93			0.665	0.649	1.083		
		7.18		///		39.06 / 39.06 ///			45.93			0.665	0.649	1.083		
VW2	Vertical	-9.67		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30		-68.58	86.61		0.665	0.877	1.465		
		-9.67		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30		-68.58	86.61		0.665	0.877	1.465		
		-9.67		-35.50 / -35.50 ///		14.92 / 14.92 ///			-68.58	86.61		0.665	1.092	1.824		
		-9.67		-35.50 / -35.50 ///		14.92 / 14.92 ///			-68.58	86.61		0.665	1.092	1.824		
VW3	Vertical	-1.58		-24.82 / -24.56 ///	1.27	0.39 / 0.65 ///	1.27		-68.58	86.61		0.665	1.461	2.466		
		-1.58		-24.82 / -24.56 ///	1.27	0.39 / 0.65 ///	1.27		-68.58	86.61		0.665	1.461	2.466		
		-1.58		-24.82 / -24.56 ///		0.39 / 0.65 ///			-68.58	86.61		0.665	1.855	3.132		
		-1.58		-24.82 / -24.56 ///		0.39 / 0.65 ///			-68.58	86.61		0.665	1.855	3.132		
VW4	Vertical	-1.58		-24.82 / -24.56 ///	1.27	0.39 / 0.65 ///	1.27		-68.58	86.61		0.665	1.461	2.466		
		-1.58		-24.82 / -24.56 ///	1.27	0.39 / 0.65 ///	1.27		-68.58	86.61		0.665	1.461	2.466		
		-1.58		-24.82 / -24.56 ///		0.39 / 0.65 ///			-68.58	86.61		0.665	1.855	3.132		
		-1.58		-24.82 / -24.56 ///		0.39 / 0.65 ///			-68.58	86.61		0.665	1.855	3.132		
VW5	Vertical	-9.67		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30		-68.58	86.61		0.665	0.877	1.465		
		-9.67		-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30		-68.58	86.61		0.665	0.877	1.465		
		-9.67		-35.50 / -35.50 ///		14.92 / 14.92 ///			-68.58	86.61		0.665	1.092	1.824		
		-9.67		-35.50 / -35.50 ///		14.92 / 14.92 ///			-68.58	86.61		0.665	1.092	1.824		
VW6	Vertical	7.18		///		39.06 / 39.06 ///	1.30		45.93			0.665	0.499	0.833		
		7.18		///		39.06 / 39.06 ///	1.30		45.93			0.665	0.499	0.833		
		7.18		///		39.06 / 39.06 ///			45.93			0.665	0.649	1.083		
		7.18		///		39.06 / 39.06 ///			45.93			0.665	0.649	1.083		
TC1	Diagonal	-32.76		-72.14 / -72.14 ///	1.20	///	///		-201.57			0.665	1.266	2.114		
		-32.76		-72.14 / -72.14 ///	1.20	///	///		-201.57			0.665	1.266	2.114		
		-32.76		-72.14 / -72.14 ///		///	///		-201.57			0.665	1.526	2.548		
		-32.76		-72.14 / -72.14 ///		///	///		-201.57			0.665	1.526	2.548		
TC7	Diagonal	-32.76		-72.14 / -72.14 ///	1.20	///	///		-201.57			0.665	1.266	2.114		
		-32.76		-72.14 / -72.14 ///	1.20	///	///		-201.57			0.665	1.266	2.114		
		-32.76		-72.14 / -72.14 ///		///	///		-201.57			0.665	1.526	2.548		
		-32.76		-72.14 / -72.14 ///		///	///		-201.57			0.665	1.526	2.548		
DW1	Diagonal	22.01		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		93.75			0.665	0.626	1.045		
		22.01		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		93.75			0.665	0.626	1.045		
		22.01		-6.92 / -6.92 ///		58.90 / 58.90 ///			93.75			0.665	0.765	1.278		
		22.01		-6.92 / -6.92 ///		58.90 / 58.90 ///			93.75			0.665	0.765	1.278		
DW2	Diagonal	11.11		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		60.00			0.665	0.555	0.926		
		11.11		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		60.00			0.665	0.555	0.926		
		11.11		-19.20 / -19.20 ///		45.67 / 45.67 ///			60.00			0.665	0.690	1.153		
		11.11		-19.20 / -19.20 ///		45.67 / 45.67 ///			60.00			0.665	0.690	1.153		
DW3	Diagonal	0.72		-0.49 / -0.81 ///	1.27	31.93 / 31.60 ///	1.27		45.93			0.665	0.768	1.296		
		0.72		-0.49 / -0.81 ///	1.27	31.93 / 31.60 ///	1.27		45.93			0.665	0.768	1.296		
		0.72		-0.49 / -0.81 ///		31.93 / 31.60 ///			45.93			0.665	0.975	1.646		
		0.72		-0.49 / -0.81 ///		31.93 / 31.60 ///			45.93			0.665	0.975	1.646		
DW4	Diagonal	0.72		-0.49 / -0.81 ///	1.27	31.93 / 31.60 ///	1.27		45.93			0.665	0.768	1.296		
		0.72		-0.49 / -0.81 ///	1.27	31.93 / 31.60 ///	1.27		45.93			0.665	0.768	1.296		
		0.72		-0.49 / -0.81 ///		31.93 / 31.60 ///			45.93			0.665	0.975	1.646		
		0.72		-0.49 / -0.81 ///		31.93 / 31.60 ///			45.93			0.665	0.975	1.646		
DW5	Diagonal	11.11		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		60.00			0.665	0.555	0.926		
		11.11		-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		60.00			0.665	0.555	0.926		
		11.11		-19.20 / -19.20 ///		45.67 / 45.67 ///			60.00			0.665	0.690	1.153		
		11.11		-19.20 / -19.20 ///		45.67 / 45.67 ///			60.00			0.665	0.690	1.153		
DW6	Diagonal	22.01		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		93.75			0.665	0.626	1.045		
		22.01		-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		93.75			0.665	0.626	1.045		
		22.01		-6.92 / -6.92 ///		58.90 / 58.90 ///			93.75			0.665	0.765	1.278		
		22.01		-6.92 / -6.92 ///		58.90 / 58.90 ///			93.75			0.665	0.765	1.278		

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: HS 20-44 (Design Lane)

SE Truss (Upstream)
Member Full Capacities

LL Scale Factor = 1.00
 Adjacent Vehicle LL Factor = 0.00
Inventory:
 A1 = 1.30, A2 = 2.17
Operating:
 A1 = 1.30, A2 = 1.30
 Note: Rating factor is outputted as 99.00 when it is greater than 99

		7.47	////		39.06 / 39.06 ///	1.30		45.93			0.665	0.494	0.825		
		7.47	////		39.06 / 39.06 ///			45.93			0.665	0.642	1.072		
		7.47	////		39.06 / 39.06 ///			45.93			0.665	0.642	1.072		
VW2	Vertical	-9.98	-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.871	1.455		
		-9.98	-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.871	1.455		
		-9.98	-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.085	1.811		
		-9.98	-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.085	1.811		
VW3	Vertical	-1.61	-24.81 / -24.54 ///	1.27	0.41 / 0.69 ///	1.27	-68.58	86.61			0.665	1.461	2.467		
		-1.61	-24.81 / -24.54 ///	1.27	0.41 / 0.69 ///	1.27	-68.58	86.61			0.665	1.461	2.467		
		-1.61	-24.81 / -24.54 ///		0.41 / 0.69 ///		-68.58	86.61			0.665	1.855	3.133		
		-1.61	-24.81 / -24.54 ///		0.41 / 0.69 ///		-68.58	86.61			0.665	1.855	3.133		
VW4	Vertical	-1.60	-24.80 / -24.52 ///	1.27	0.40 / 0.67 ///	1.27	-68.58	86.61			0.665	1.462	2.469		
		-1.60	-24.80 / -24.52 ///	1.27	0.40 / 0.67 ///	1.27	-68.58	86.61			0.665	1.462	2.469		
		-1.60	-24.80 / -24.52 ///		0.40 / 0.67 ///		-68.58	86.61			0.665	1.856	3.135		
		-1.60	-24.80 / -24.52 ///		0.40 / 0.67 ///		-68.58	86.61			0.665	1.856	3.135		
VW5	Vertical	-9.95	-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.872	1.456		
		-9.95	-35.50 / -35.50 ///	1.24	14.92 / 14.92 ///	1.30	-68.58	86.61			0.665	0.872	1.456		
		-9.95	-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.085	1.812		
		-9.95	-35.50 / -35.50 ///		14.92 / 14.92 ///		-68.58	86.61			0.665	1.085	1.812		
VW6R	Vertical	7.51	////		39.06 / 39.06 ///	1.30		49.38			0.665	0.540	0.902		
		7.51	////		39.06 / 39.06 ///	1.30		49.38			0.665	0.540	0.902		
		7.51	////		39.06 / 39.06 ///			49.38			0.665	0.702	1.173		
		7.51	////		39.06 / 39.06 ///			49.38			0.665	0.702	1.173		
TC1	Diagonal	-33.90	-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.254	2.095		
		-33.90	-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.254	2.095		
		-33.90	-72.14 / -72.14 ///		////	////	-201.57				0.665	1.511	2.524		
		-33.90	-72.14 / -72.14 ///		////	////	-201.57				0.665	1.511	2.524		
TC7	Diagonal	-33.96	-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.254	2.094		
		-33.96	-72.14 / -72.14 ///	1.20	////	////	-201.57				0.665	1.254	2.094		
		-33.96	-72.14 / -72.14 ///		////	////	-201.57				0.665	1.511	2.523		
		-33.96	-72.14 / -72.14 ///		////	////	-201.57				0.665	1.511	2.523		
DW1	Diagonal	22.77	-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		93.75			0.665	0.616	1.029		
		22.77	-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		93.75			0.665	0.616	1.029		
		22.77	-6.92 / -6.92 ///		58.90 / 58.90 ///			93.75			0.665	0.754	1.259		
		22.77	-6.92 / -6.92 ///		58.90 / 58.90 ///			93.75			0.665	0.754	1.259		
DW2	Diagonal	11.50	-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		60.00			0.665	0.549	0.916		
		11.50	-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		60.00			0.665	0.549	0.916		
		11.50	-19.20 / -19.20 ///		45.67 / 45.67 ///			60.00			0.665	0.683	1.140		
		11.50	-19.20 / -19.20 ///		45.67 / 45.67 ///			60.00			0.665	0.683	1.140		
DW3	Diagonal	0.76	-0.49 / -0.87 ///	1.27	31.92 / 31.57 ///	1.27		45.93			0.665	0.768	1.296		
		0.76	-0.49 / -0.87 ///	1.27	31.92 / 31.57 ///	1.27		45.93			0.665	0.768	1.296		
		0.76	-0.49 / -0.87 ///		31.92 / 31.57 ///			45.93			0.665	0.975	1.646		
		0.76	-0.49 / -0.87 ///		31.92 / 31.57 ///			45.93			0.665	0.975	1.646		
DW4	Diagonal	0.75	-0.49 / -0.86 ///	1.27	31.91 / 31.55 ///	1.27		45.93			0.665	0.768	1.297		
		0.75	-0.49 / -0.86 ///	1.27	31.91 / 31.55 ///	1.27		45.93			0.665	0.768	1.297		
		0.75	-0.49 / -0.86 ///		31.91 / 31.55 ///			45.93			0.665	0.975	1.647		
		0.75	-0.49 / -0.86 ///		31.91 / 31.55 ///			45.93			0.665	0.975	1.647		
DW5	Diagonal	11.47	-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		60.00			0.665	0.549	0.917		
		11.47	-19.20 / -19.20 ///	1.30	45.67 / 45.67 ///	1.24		60.00			0.665	0.549	0.917		
		11.47	-19.20 / -19.20 ///		45.67 / 45.67 ///			60.00			0.665	0.683	1.141		
		11.47	-19.20 / -19.20 ///		45.67 / 45.67 ///			60.00			0.665	0.683	1.141		
DW6	Diagonal	22.75	-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		93.75			0.665	0.617	1.030		
		22.75	-6.92 / -6.92 ///	1.30	58.90 / 58.90 ///	1.22		93.75			0.665	0.617	1.030		
		22.75	-6.92 / -6.92 ///		58.90 / 58.90 ///			93.75			0.665	0.754	1.260		
		22.75	-6.92 / -6.92 ///		58.90 / 58.90 ///			93.75			0.665	0.754	1.260		

LLDF	Single Lane	Multi Lane
Force	0.665	0.665
Deflection	0.500	0.500

Live Load: HS 20-44 (Design Lane)

BRIDGE RATING INFORMATION SHEET – DECKS

Bridge ID: *L15672000+02001*

Year Constructed: *1911*

Feature Intersected: *Swan River*

Location: *S End Big Fork*

Date Submitted: *12/5/14*

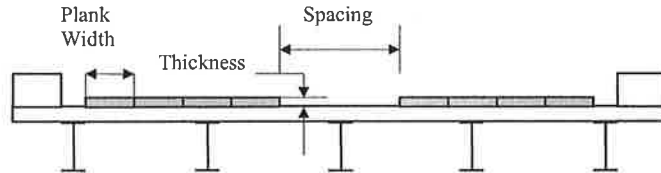
County: *Flathead*

District/Division: *Missoula*

Inspector: *BBL/PKS
Stelling Engineers, Inc.
(KLS)*

Deck Material:

Timber:



Deck Species: Larch/Fir Glu-Lam Other (specify):

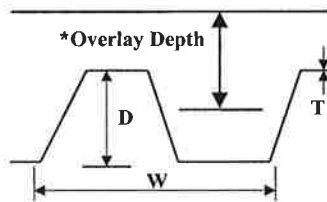
Conditions: Dry Wet

Type (Nail-Lam, Glu-Lam, or Plank)	Board Width (in)	Board Thickness (in)	Running Planks? (Y or N)	Running Plank Width (in)	Running Plank Thickness (in)	Number of Running Planks Left	Number of Running Planks Right	Running Plank Spacing
<i>Plank</i>	<i>9 1/2 → 12</i>	<i>4</i>	<i>Y</i>	<i>9 1/2</i>	<i>2 1/2</i>	<i>Full Deck Width</i>		

Concrete: Reinforced Cast-in-Place Reinforced Precast Panels Prestressed Precast Panels Unknown

Concrete Deck Thickness (in):

Corrugated Metal Decking:



D (in) =

T (in) =

W (in) =

Overlay Depth (in) =

Other (specify): Thickness (in) =

Overlay? Yes No Material = Thickness (in) =

Curb? Yes No

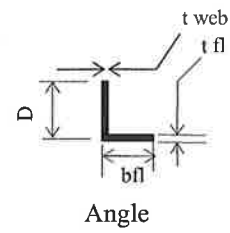
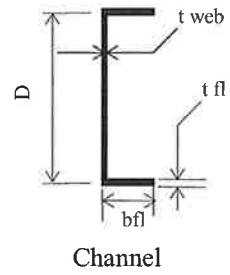
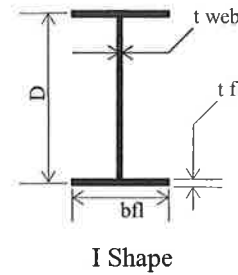
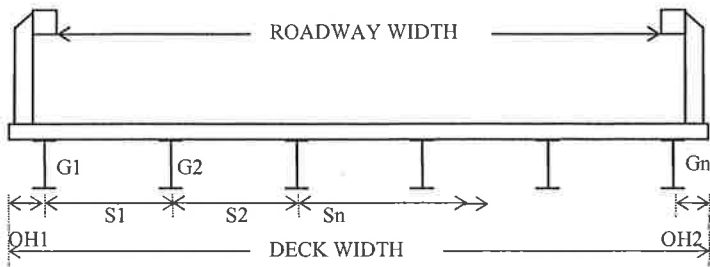
Material = Top Width (in) = Bottom Width (in) = Height (in) =

Guardrail? Yes No Type = *W-rail* Height (ft) = *3'-5"* Post Material = *Steel*

Comments:

Note: Use the back of this sheet for sketches.

SPAN CROSS-SECTION - STEEL GIRDERS



Span No: 1 (End Spans)

Roadway Width (Curb-to-Curb or Face to Face of Rail): 15 ft 7 1/2 in

Deck Width: 16 ft 1 in Number of Girders: 8

Deck Overhang: OH1: 3 1/2 in OH2: 3 1/2 in

Girder Material: Plain Steel Weathering Steel

Note: Girder shapes can be I shapes, C for Channel shapes, A for Angle Shapes, or O for built-up and strange shapes. Provide a sketch if you enter an O shape. O should also be used for shapes with variable cross-sections (variable girder depth, etc).

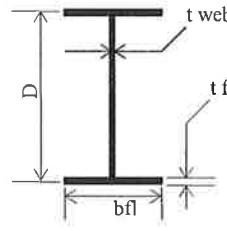
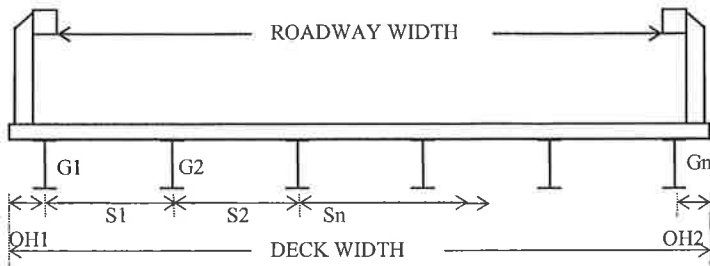
Girder Cross-Section	Shape (I, C, A, or O)	D (depth) (in)	bfl (flange width) (in)	tfl (flange thickness) (in)	tweb (web thickness) (in)
A	C 8x11.5	8	2 5/16	0.46	
B	W 8x15	8	4 1/8	0.33	
C					
D					
E					

Girder	Cross-Section	Spacing (ft)
G1	A	2'-9"
G2	B	2'-0"
G3	B	2'-0"
G4	B	2'-0"
G5	B	2'-0"
G6	B	2'-0"
G7	B	2'-9"

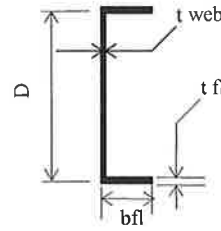
Girder	Cross-Section	Spacing (ft)
G8	A	
G9		
G10		
G11		
G12		
G13		
G14		

Comments:

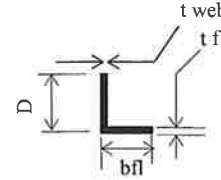
ADDITIONAL SPAN CROSS-SECTION - STEEL GIRDERS



I Shape



Channel



Angle

Span No: 2-6 (Interior Spans)

Roadway Width (Curb-to-Curb or Face to Face of Rail): 15 ft 7 1/2 in

Deck Width: 16 ft 1 in

Number of Girders: 8

Deck Overhang: OH1: 3 1/2 in OH2: 3 1/2 in

Girder Material: Plain Steel Weathering Steel

Note: Girder shapes can be I shapes, C for Channel shapes, A for Angle Shapes, or O for built-up and strange shapes. Provide a sketch if you enter an O shape. O should also be used for shapes with variable cross-sections (variable girder depth, etc).

Girder Cross-Section	Shape (I, C, A, or O)	D (depth) (in)	bfl (flange width) (in)	tfl (flange thickness) (in)	tweb (web thickness) (in)
A	C 8x11.5	8	2 5/16	0.46	
B	S 8x18.4	8	4		
C					
D					
E					

Girder	Cross-Section	Spacing (ft)
G1	A	2'-9"
G2	B	2'-0"
G3	B	2'-0"
G4	B	2'-0"
G5	B	2'-0"
G6	B	2'-0"
G7	B	2'-9"

Girder	Cross-Section	Spacing (ft)
G8	A	
G9		
G10		
G11		
G12		
G13		
G14		

Comments:

BRIDGE MEASUREMENT FORM – SIMPLE SPAN STEEL GIRDER

Bridge ID: *L15672000 + 02001*

Year Constructed: *1911*

Feature Intersected: *Swan River*

Location: *S End Big Fork*

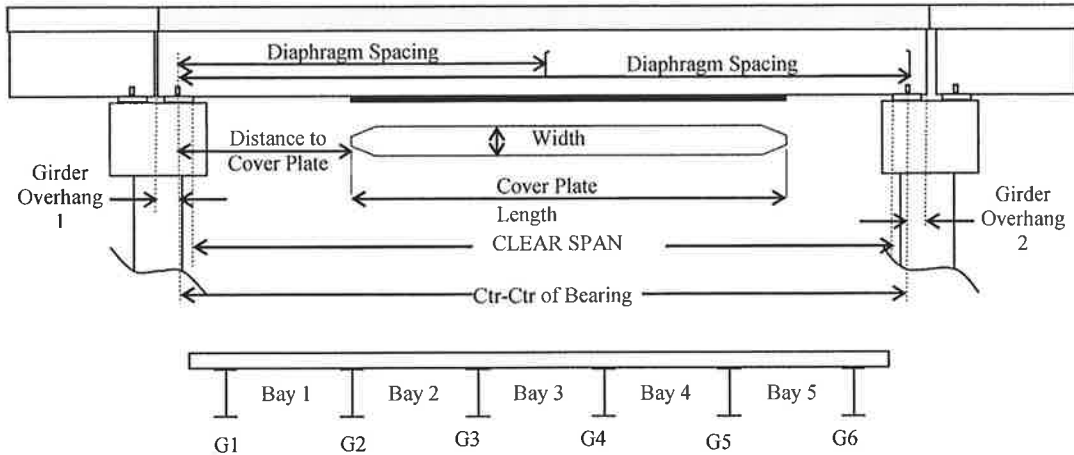
Date Submitted: *12/5/14*

County: *Flathead*

District/Division: *Missoula*

Inspector: *BBL / PKS*
Stelling Engineers, Inc. (KLJ)

STEEL SIMPLE SPAN LENGTH DEFINITIONS



Girders and girder bays are numbered from left to right while looking ahead on line.

Diaphragms? Yes No

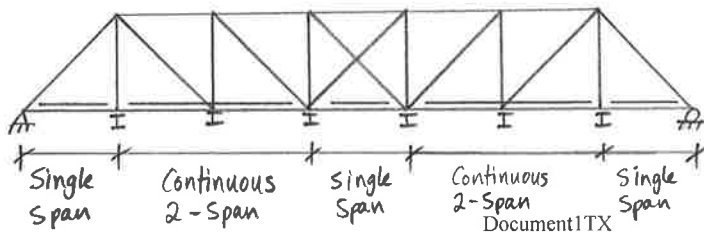
Span Number	Span Length (ctr to ctr of bearing) (ft)	Clear Span (ft)	Girder Overhang 1 (in)	Girder Overhang 2 (in)	Skew 1 (degrees)	Skew 2 (degrees)
<i>1, 4, 7</i>	<i>17'-0"</i>	<i>16'-6 1/2"</i>	<i>1 3/8"</i>	<i>1 3/8"</i>	<i>0</i>	<i>0</i>

Span	Girder Bay(s)	Diaphragm Spacing (ahead on line) (ft)

Cover Plates? Yes No

Span	Girder(s)	Distance to Cover Plate (ft)	Length of Cover Plate (ft)	Width of Cover Plate (in)	Thickness of Cover Plate (in)

Comments:



Assumed stringer configuration, as viewed from Abutments.

BRIDGE MEASUREMENT FORM – CONTINUOUS STEEL GIRDER

Bridge ID: L15672000 + 02001

Year Constructed: 1911

Feature Intersected: Swan River

Location: S End Big Fork

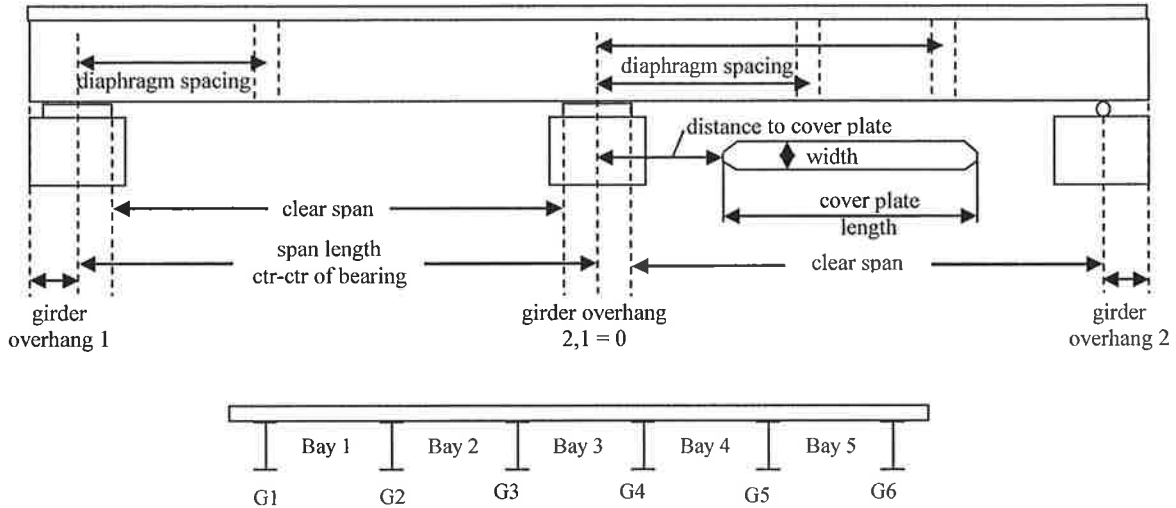
Date Submitted: 12/5/14

County: Flathead

District/Division: Missoula

Inspector: BBL / PKS
Stelling Engineers, Inc. (KLJ)

CONTINUOUS STEEL GIRDER SPAN LENGTH DEFINITIONS



Girders and girder bays are numbered from left to right while looking ahead on line.

Diaphragms? Yes No

Span Number	Span Length (ctr to ctr of bearing) (ft)	Clear Span (ft)	Girder Overhang 1 (in)	Girder Overhang 2 (in)
2-3	17' x 2 = 34'	16'-6 1/2"	1 3/8"	1 3/8"
5-6	17' x 2 = 34'	16'-6 1/2"	1 3/8"	1 3/8"

Diaphragm Spacing:

Span	Girder Bay(s)	Diaphragm Spacing (ahead on line) (ft)

Cover Plates? Yes No

Span	Girder(s)	Distance to Cover Plate (ft)	Length of Cover Plate (ft)	Width of Cover Plate (in)	Thickness of Cover Plate (in)